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AMERICAN BRIDGE COMPANY

STANDARDS

FOR

STRUCTURAL DETAILS

1901

ENGINEERING DEPARTMENT,

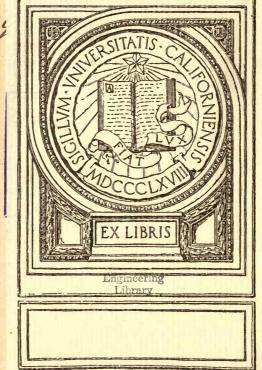
C. G. Schneider, Vice-President

Paul L. Wolfel, Chief Engineer UNIVERSITY OF FORMIA,

Bean, College of Civil Eng.

The lewil Engineering Association of the huiversity of lealifornia, with the compliments of fank onle.

Nov. 16 - 15



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ERRATA.

Page 38, second formula, in left hand column, should read $y - \sqrt{(a+at)^2 + \overline{ht}^2}$.

Page 70, second line, should read "the black which is to be painted" instead of "the flat which is to be painted."

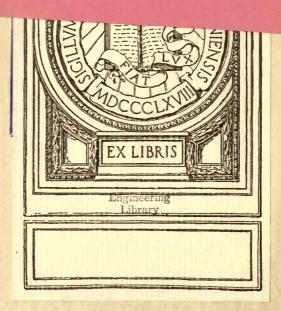
Page 85, eleventh line, should read "uneven number of panels" instead of "even number of panels."

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DEPARTMENT OF CIVIL ENGINEERING
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UNIVERSITY OF FORNIA,

Dean, College of Civil Eng.

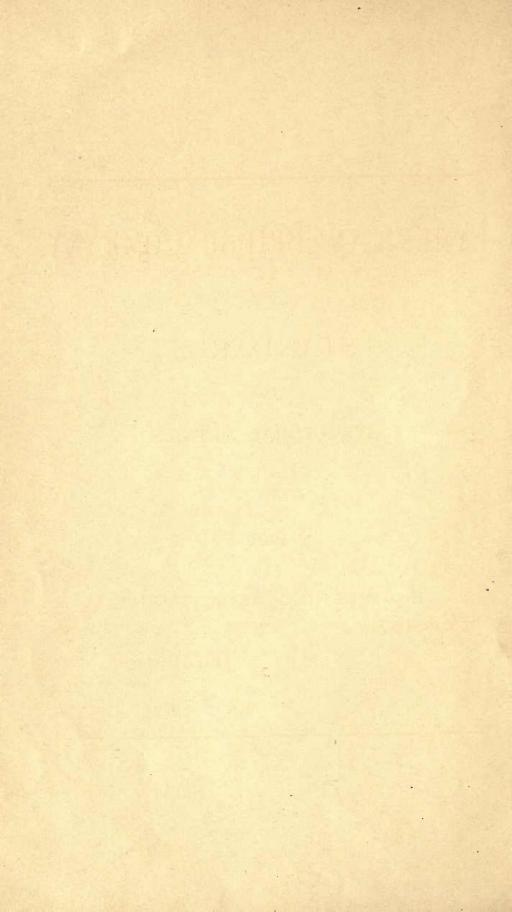
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STANDARDS

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STRUCTURAL DETAILS

1901

ENGINEERING DEPARTMENT,

C. C. SCHNEIDER,

Vice-President

PAUL L. WOLFEL, Chief Engineer

TAGE TAGE TO THE STATE OF THE S

Engineering Library NOTE.

All shapes are those manufactured by the A. & P. Roberts Co. Pencoyd Iron Works.

For Carnegie Steel Co.'s shapes see Appendix.

PREFACE.

In order to obtain uniformity in the work done at the various plants of the American Bridge Company, it has been deemed advisable to prepare a system of standards for use in every engineering office to assist the engineers and draughtsmen in making detail and shop drawings.

These standards are the result of years of experience. They have been revised from time to time in order to keep pace with the progress made in the art of designing, and particular attention has been paid to have them adapted to the latest improvements in tools used in bridge construction. They also contain such useful tables and information as will be found convenient in every engineering office where steelwork is being designed.

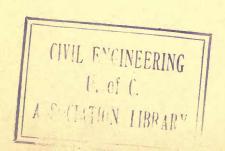
This present edition is a revision of former standards. Before finally adopting the same as the standards of the American Bridge Company, the engineers of the different plants were consulted and their suggestions incorporated, so that these standards, as now presented, are applicable to steel structures of all kinds.

Pencoyd, September, 1901.

C. C. SCHNEIDER, Vice-President.

> Paul L. Wolfel, Chief Engineer.

793205



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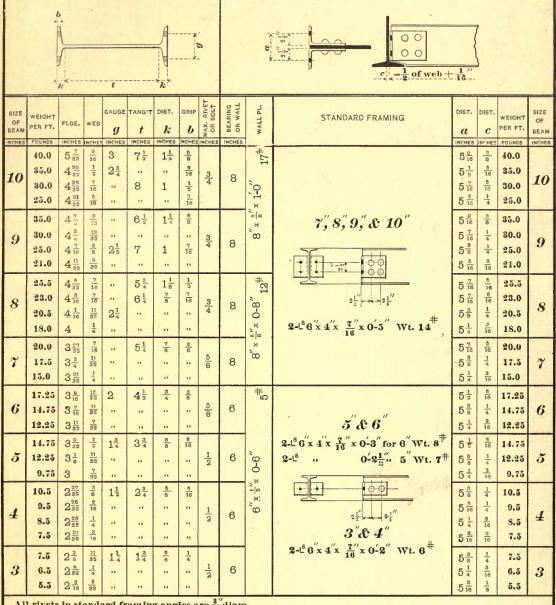
BEAMS.
Weights, dimensions, framing etc., etc.

	ot: A					1	8			2 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	k			t		k					$e = \frac{1}{2} \text{ of web} + \frac{1}{16}''$
SIZE OF BEAM	WEIGHT PER FOOT	FLGE.	WEB	g	TANG'T	bist.		MAX. RIVET OR BOLT.	BEARING ON WALL	WALL PL.	STANDARD FRAMING DIST. DIST. WEIGHT SIZE OF PER FOOT BEAN
INCHES	100.0 95.0	$7\frac{1}{4}$ $7\frac{3}{16}$	3 4 11 16	4	2034	15 8	7 8	NCHE9	INCHES	1-4# 73#	24"
*24	90.0	$7\frac{1}{8}$ $7\frac{1}{16}$	16 5 8 9 16	66	66	66.0		1	16	, 1 , x	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	80.0	$\frac{7}{7}$	1 2 7 8	44	16 1/4	17/8	15 16			73# 16	5 80.0
	95.0 90.0	$6\frac{16}{18}$ $6\frac{7}{8}$	25 32 23 32	£ 6	"	"	16			1-4"	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$
20	85.0 80.0	$6\frac{25}{32}$ $6\frac{3}{4}$	11 18 5 8		16½	1,4	7 8	7 8	16	×××××××××××××××××××××××××××××××××××××××	511 7 16 85.0
	75.0 70.0	6 3/4 6 9/32	5 8 0 16	66	17	1 1 1 2	3 4			16"×	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	90.0	6 1/4	1 2	3 3 4		(1	11 16			73 [#]	51 6 65.0
	85.0 80.0			64						1-4"	80.0
18	75.0	$6\frac{23}{32}$ $6\frac{1}{2}$	21 32 9 16	14	14 1/2	13/4	13 16 3 4	7 8	16	16"× 1"×	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	65.0 60.0 55.0	$6\frac{1}{2}$ $6\frac{7}{32}$	9 18 17 32 15 32	44	15	1 1 1 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	11 16			16	$2 + 1^{\frac{1}{6}} 4^{\frac{1}{8}} 4^{\frac{1}{8}} 4^{\frac{1}{8}} 1^{\frac{1}{3}} 4^{\frac{1}{3}} \mathbf{Wt. 31}^{\frac{1}{3}} \begin{bmatrix} 5\frac{0}{16} & \frac{3}{8} & 65.0 \\ 5\frac{0}{16} & \frac{3}{8} & 60.0 \\ 5\frac{1}{2} & \frac{5}{16} & \frac{5}{5}.0 \end{bmatrix}$
	80.0	$6\frac{5}{8}$ $6\frac{17}{32}$	27 32 23 32	44	11 1/2	13/4	18 16			#4	$15'' \qquad \begin{array}{c ccccccccccccccccccccccccccccccccccc$
	70.0 65.0	$6\frac{7}{16}$ $6\frac{9}{32}$	5 8 21 32	66	" 11 3/4	1 5 8	3 4			× 1-4"	5 8 3 70.0 5 1 10 8 65.0
15	60.0 55.0	$6\frac{1}{8}$ $6\frac{1}{8}$	1 2	$3\frac{1}{2}$	12	1 1 1 2	11 16	3 4	12	12"× ¾"	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	50.0 45.0	$5\frac{13}{18} \\ 5\frac{17}{32}$	1 2 7 16	44	12½	1 1 1 4	9 16				$2-\frac{16}{6} \frac{6}{4} \times \frac{4}{4} \times \frac{2}{16} \times 0^{-1}0^{4} \text{Wt. } 27 \qquad 5\frac{1}{2} \frac{8}{16} 50.0 \\ 5\frac{7}{16} \frac{8}{16} 45.0 \\ 5\frac{7}{16} \frac{1}{4} 42.0 $
	65.0	5 ½ 6	13 32 13 16	66	8 3/4	15/8	7 8			31#	12"
	60.0 55.0	5 ½ 5 ¾ 5 ¼	11 16 9 16	6.6	**	11	"			1-0″	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
12	50.0 45.0 40.0	5 ½ 5 3 8 5 ½ 4	9 18 9 18 7 16	3	9 9 1/4	13/8	3 4 11 16	3 4	34 12 2	2"x 4",	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	35.0 31.5	$5\frac{3}{32}$	7 16 11 32	66	9 3 4	1 1 8	1 2			1;	$2-\frac{1}{6}\frac{6}{6}\times\frac{4}{4}\times\frac{7}{16}\times\frac{7}{16}\times\frac{7}{2}$ 1000 $\frac{5}{7}\frac{7}{16}$ $5\frac{7}{16}$ $5\frac{7}{16}$ $5\frac{7}{16}$ $5\frac{7}{16}$ $5\frac{7}{16}$ 35.0 $5\frac{8}{14}$ 31.5
* 0	arnegie	e Steel	Co.	s Sect							1

^{*} Carnegie Steel Co.'s Sect.

BEAMS

Weights, dimensions, framing etc., etc.



All rivets in standard framing angles are $\frac{3}{4}$ diam.

Weights of include weight of shop rivets,

When beams frame opposite each other into another beam with web thickness less than $\frac{y}{10}$ or where beams of short span lengths are loaded to their full capacity, it may be necessary to use framing angles of greater strength than the standards.

See table below for minimum span lengths.

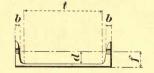
SIZE	WEIGHT	SPAN IN																		
24	80.0	22.0	18	70.0	16.5	15	70.0	18.0	12	50.0	140	10	35.0	12.0	8	18.0	5.5	5	9.75	4.0
					14.0															
	65.0	180				11	42.0	11.0	11	31.5	9.0	9	21.0	7.0	6	12.25	6.0	3	5.5	2.0

CHANNELS.
Weights dimensions framing etc., etc.

	b	k		t	7		å [*]		a	f	This leg pun as 6"leg of s	ched same	-Wet	
SIZE OF CHANNEL	SO WEIGHT	FLANGE	.s.	S GAUGE	TANG.	K INS.	D GRIP	RIVET OR BOLT.	TSIG d	B CHES	. INCHES	C INCHES	WEIGHT SO PER FT.	SIZE OF CHANNEL
15	55.0 50.0 45.0 40.0 35.0 33.0	$\begin{array}{c} 4\frac{3}{32} \\ 4\\ 3\frac{5}{8} \\ 3\frac{17}{32} \\ 3\frac{18}{32} \\ 3\frac{18}{32} \end{array}$	23 32 5 8 17 32 7 16 13 32	$2^{\frac{1}{2}}$ " 2 " "	12 " 12 ¹ / ₄ "	1 ½ '' 1 3 6 '' '' '' '' ''	3 4 21 5 B 21 11 11 11 11 11 11	34	1 ³ / ₄ ,, 1 ⁵ / ₆ ,, ,,	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3 \frac{1}{4} 3 \frac{1}{6} 3 \frac{1}{6} 3 \frac{1}{16} 2 \frac{15}{16} 2 \frac{15}{16}	19 16 11 10 11 16 5 8 1 2 1 2	55.0 50.0 45.0 40.0 35.0 33.0	15
12	40.0 35.0 30.0 25.0 20.5	$3\frac{5}{16}$ $3\frac{8}{16}$ $3\frac{5}{32}$ $3\frac{1}{16}$ $2\frac{15}{16}$	5 8 1 2 11 13 32 9 32	1; 1 ³ / ₄	9 " 10 " " "	1 ¹ / ₂ ,, 1	3 4 11 1 2 11	3 4	1 ¹ / ₂ ,, 1 ³ / ₈ ,,	$\begin{array}{c} 2\frac{1}{8} \\ 2 \\ 1\frac{7}{8} \\ 1\frac{25}{32} \\ 1\frac{21}{32} \end{array}$	3 ¹ / _e 3 3 2 ¹⁸ / ₁₆ 2 ¹⁹ / ₁₆	11 0 16 0 16 16 12 2 3	40.0 35.0 30.0 25.0 20.5	12
10	\$5.0 30.0 25.0 20.0 15.0	$\begin{array}{c} 3\frac{7}{32} \\ 3\frac{1}{16} \\ 2\frac{29}{32} \\ 2\frac{11}{16} \\ 2\frac{9}{16} \end{array}$	3 4 19 32 7 16 3 8 1 4	1: 1: 1 ¹ / ₂	7 ³ / ₄ " 8 "	1 1 8 · · · · · · · · · · · · · · · · ·)) 11 11 17 16 11	3 4	1 ¹ / ₄ ,, ,, ,,	$\begin{array}{c} 2 \\ 1\frac{27}{32} \\ 1\frac{11}{18} \\ 1\frac{5}{6} \\ 1\frac{1}{2} \end{array}$	$\begin{array}{c} 3^{\frac{1}{4}} \\ 3^{\frac{1}{8}} \\ 2^{\frac{15}{16}} \\ 2^{\frac{7}{9}} \\ 2^{\frac{3}{4}} \end{array}$	13 16 11 10 1 2 7 16 5	35.0 30.0 25.0 20.0 15.0	10
9	25.0 20.0 15.0 13.25	$\begin{array}{c} 2\frac{7}{8} \\ 2\frac{23}{32} \\ 2\frac{1}{2} \\ 2\frac{7}{16} \end{array}$	17 32 3 8 9 32 1 4	13 18	7,, 7 ¹ / ₄	11 7 6	1 2 2 7 16	34	1 ¹ / ₆	$1\frac{21}{32}$ $1\frac{1}{2}$ $1\frac{13}{32}$ $1\frac{3}{8}$	$\begin{array}{c} 3\frac{1}{16} \\ 2\frac{7}{8} \\ 2\frac{13}{16} \\ 2\frac{13}{4} \\ \end{array}$	5 6 7 16 3 6 5 16	25.0 20.0 15.0 13.25	9
8	21.25 18.75 16.25 13.75 11.25	$\begin{array}{c} 2\frac{3}{4} \\ 2\frac{21}{32} \\ 2\frac{9}{18} \\ 2\frac{11}{32} \\ 2\frac{1}{4} \end{array}$	17 32 7 16 11 32 5 16 7 32	1½ · · · · · · · · · · · · · · · · · · ·	6 ,, 6 ¹ / ₄	1 " " 7 6 "	11 11 3 8	3 4	,, ,, 1 1 16	$ \begin{array}{c} 1\frac{21}{32} \\ 1\frac{9}{16} \\ 1\frac{15}{32} \\ 1\frac{3}{6} \\ 1\frac{9}{32} \end{array} $	$\begin{array}{c} 3\frac{1}{16} \\ 2\frac{15}{16} \\ 2\frac{7}{8} \\ 2\frac{7}{16} \\ 2\frac{13}{16} \\ 2\frac{3}{4} \end{array}$	8 6 7 7 16 8 5 16	21.25 18.75 16.25 13.75 11.25	8
7	19.75 17.25 14.75 12.25 9.75	$\begin{array}{c} 2\frac{21}{32} \\ 2\frac{9}{16} \\ 2\frac{7}{16} \\ 2\frac{3}{16} \\ 2\frac{3}{32} \end{array}$	9 16 15 32 3 6 5 16 7	1½ " " 1¼ " " " " " " " " " " " " " " " "	5 ¹ / ₄	11 11 3 4	7 18 11	5 8	22 23 20 22 22	$ \begin{array}{c} 1\frac{8}{8} \\ 1\frac{17}{32} \\ 1\frac{7}{16} \\ 1\frac{3}{8} \\ 1\frac{9}{32} \end{array} $	$\begin{array}{c} 3\frac{1}{10} \\ 3 \\ 2\frac{7}{8} \\ 2\frac{19}{16} \\ 2\frac{3}{4} \end{array}$	5 8 9 16 7 10 9 8	19.75 17.25 14.75 12.25 9.75	7
6	15.50 13.00 10.50 8.00	2 d d d d d d d d d d d d d d d d d d d	17 32 13 32 2 33 33 31 3	11 1 1 1 8	41/2	11 11 11	11 11 11 5 16	5/8	1,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	$ \begin{array}{c} 1\frac{17}{32} \\ 1\frac{13}{32} \\ 1\frac{3}{32} \\ 1\frac{3}{16} \end{array} $	$egin{array}{c} 3^{rac{1}{16}} \ . & 2^{rac{15}{16}} \ . & 2^{rac{19}{16}} \ . & 2^{rac{11}{16}} \ . & 2^{r$	5 6 123 8 14	15.50 13.00 10.50 8.00	6
5	11.50 9.00 6.50	$\begin{array}{c} 2\frac{1}{32} \\ 1\frac{7}{8} \\ 1\frac{3}{4} \end{array}$	15 32 5 16 3 16	11	3 1/2	79 77 77	11	1/2	7 6 11	$ \begin{array}{c} 1\frac{11}{32} \\ 1\frac{3}{16} \\ 1\frac{1}{16} \end{array} $	$\frac{3}{2^{\frac{13}{16}}}$	9 16 3 8 1	11.50 9.00 6.50	5
4	7.25 6.25 5.25	$\begin{array}{c} 1\frac{25}{32} \\ 1\frac{11}{16} \\ 1\frac{5}{6} \end{array}$	11 32 1 4 3 16	1, ,,	2 3 4 77	5 8 11	1 4 11	1/2	3/4 11	1 3/2 1 1 15/16	$2\frac{7}{8}$ $2\frac{3}{4}$ $2\frac{11}{16}$	7 16 35 16 14	7.25 6.25 5.25	4
3	6.0 5.0 .4.0	$\begin{array}{c c} 1\frac{19}{32} \\ 1\frac{1}{2} \\ 1\frac{13}{32} \end{array}$	11 32 1 4 5 32	7 8	1 3 4 11	>2 >> >>	"	1/2	5 6 22	31 32 7 8 25 32	$\begin{array}{c} 2\frac{7}{8} \\ 2\frac{3}{4} \\ 2\frac{11}{16} \end{array}$	7. 16 5. 16 14	6.0 5.0 4.0	3

CHANNELS.

Weights, Areas, Dimensions, etc., etc.



												_								-	
SIZE OF CHANNEL	SECTION	AREA	FLANGE	WEB	WEIGHT PER FT.	d DIST.	J. GAUGE	TANGT	P GRIP	MAX, RIVET OR BOLT	MAX. RIVET OR BOLT	Q GRIP	TANG'T	J. GAUGE	d DIST.	WEIGHT PER FT.	WEB	FLANGE	AREA	SECTION	SIZE OF CHANNEL
INS.		SQ. INS.	INS.	INS.	LBS.	INS.	INS.	INS.	INS.	INS.	INS.	INS.	INS.	INS.	INS.	LBS.	INS.	INS.	SQ. INS.		INS.
		20.3	4 3 8	1	69.0		23/4							2		35.1	3 4	3 7 32	10.3		- 3
		19.4	4 5 16	15 16	65.8		211	0						1 15 16		83.0	11 16	3 5 32	9.7		17.19
4 E	>	18.4	$4^{\frac{1}{4}}$	7 6	62.7		2 ⁵ / _B							17/8		30.9	5 8	3 3 3 2	9.1	>	
	HEAVY	17.5	$4\frac{3}{16}$	13	59.5	13/4	200	12	34	7 8	$\frac{3}{4}$	$\frac{1}{2}$	$7\frac{3}{4}$	113	$1\frac{1}{4}$	28.7	9 16	3 1 32	8.5	HEAVY	
	Ī	16.6	$4\frac{1}{6}$	3	56.3		$2\frac{1}{2}$							$1\frac{3}{4}$		26.6	1/2	$2\frac{31}{32}$	7.8	_	
		15.6	4 1/16	11 16	53.1		$2\frac{7}{16}$	- 4						111		24.5	7 16	$2\frac{29}{32}$	7.2	- 14	10
15		14.8	4	5 8	50.0		23/8							11116		22.0	7 16	$2\frac{25}{32}$	6.5		
Less.		14.3	31110	11/16	48.6	150	$2\frac{5}{16}$					-		1 5		20.0	3 8	$2\frac{23}{32}$	5.9	-	
		13.4	3 = 8	5 8	45.0		$2\frac{1}{4}$				$\frac{3}{4}$	7 18	8	1 9 16	11/4	17.8	5 16	$2\frac{21}{32}$	5.2	LIGHT	- 1
	H	12.4	$3\frac{9}{16}$	9 16	42.2	45	23	101	5	7				1 1 2		15.0	1/4	$2\frac{19}{32}$	4.5	Ĭ	
	LIGHT	11.5	31/2	1 2	39.0	15/8	$2\frac{1}{8}$	$12\frac{1}{4}$	8	7 8											
		10.5	37	7 18	35.9		$2\frac{1}{16}$							113		29.4	11 18	31/32	8.7		
		9.6	3 3 3 3	13 32	33.0		$2\frac{1}{32}$							13/4		27.5	8	$2\frac{31}{32}$	8.1		
		14.3	4 3 8	3 4	48.5		2 ₺							111		25.6	9 16	$2\frac{29}{32}$	7.5	>	
		13.5	$4\frac{5}{16}$	11 16	45.7		2 2 0				$\frac{3}{4}$	$\frac{1}{2}$	7	1 5 8	1 1 8	23.7	1/2	237	7.0	HEAVY	1 3
		12.6	4 1/4	5 8	42.9		$2\frac{1}{2}$			2				1 9 16		21.8	7 18	$2\frac{25}{32}$	6.4	_	0
13		11.8	$4\frac{3}{16}$	9 16	40.2	17/6	2716	101	5 8	78	_ ,,			11/2		19.9	3 6	$2\frac{23}{32}$	5.8		9
bei i		11.0	4 1 6	1/2	37.4		$2\frac{3}{6}$							$1\frac{1}{2}$		17.8	3 8	$2\frac{19}{32}$	5.2	7, 6	
		10.2	$4\frac{1}{16}$	7 16	84.7		$2\frac{5}{16}$				3	7	_,	$1\frac{7}{16}$		15.9	5 16	$2\frac{17}{32}$	4.7	보	
		9.4	4	3 8	31.9		$2\frac{1}{4}$				$\frac{3}{4}$	7 16	$7\frac{1}{4}$	1 3	11/8	14.0	1 4	$2\frac{15}{32}$	4.1	LIGHT	
		16.3	311/16	1	55.5		$2\frac{1}{2}$				7			1 11 32		13.25	7 32	$2\frac{7}{16}$	3.0	_	
		15.6	3 8	15 16	52.9		$2\frac{7}{16}$							1.5		20.8	1/2	$2\frac{23}{32}$	6.1		
		14.8	3 9 16	7 8	50.4		$2\frac{3}{8}$							1 1 1 6		19.1	7 16	$2\frac{21}{32}$	5.6	>	
		14.1	31/2	13 10	47.8		25				$\frac{3}{4}$	$\frac{7}{16}$	6	$1^{\frac{1}{2}}$	1 1 6	17.4	3 8	$2\frac{19}{32}$	5.1	HEAVY	
	\\ \	13.3	3 7/16	3 4	45.3	41	$2\frac{1}{4}$		3	7				1 7 18		15.7	5 16	$2\frac{17}{32}$	4.6	_	8
	HEA	12.6	3 8	11	42.7	11/2	2316	9	3 4	7 8				1 3 6		14.0	5 18	$2\frac{11}{32}$	4.1	-	
		11.8	$3\frac{5}{16}$	5 6	40.2		21/8				$\frac{3}{4}$	3/8	$6\frac{1}{4}$	15	110	12.3	1/4	$2\frac{9}{32}$	3.6	LIGHT	
12		11.1	31/4	9 10	37.6		$2\frac{1}{16}$							1 3 2		11.25	7 32	$2\frac{1}{4}$	3.4	7	100
		10.3	$3\frac{3}{16}$	1/2	35.1		2											-			
		9.6	$3\frac{1}{8}$	716	32.5		1 18														1
			- B	1			17					Ti	ese W	eigh	ts ar	e used	iin	Brld	ge Wo	ork	

These weights are used in Bridge Work for chords, posts, etc. When ordering from Mill give weight

and section of [

1 2

 $3\frac{5}{32} \frac{1}{2}$

6.4 231

 $3\frac{1}{32}$ $\frac{3}{8}$

5 21.8

29.4

24.3 $1\frac{3}{6}$ $1\frac{3}{4}$ 10

1 7 8

 $1\frac{13}{16}$

 $1\frac{11}{16}$

131

8.7

7.9 $3\frac{3}{39}$ $\frac{7}{16}$ 26.9

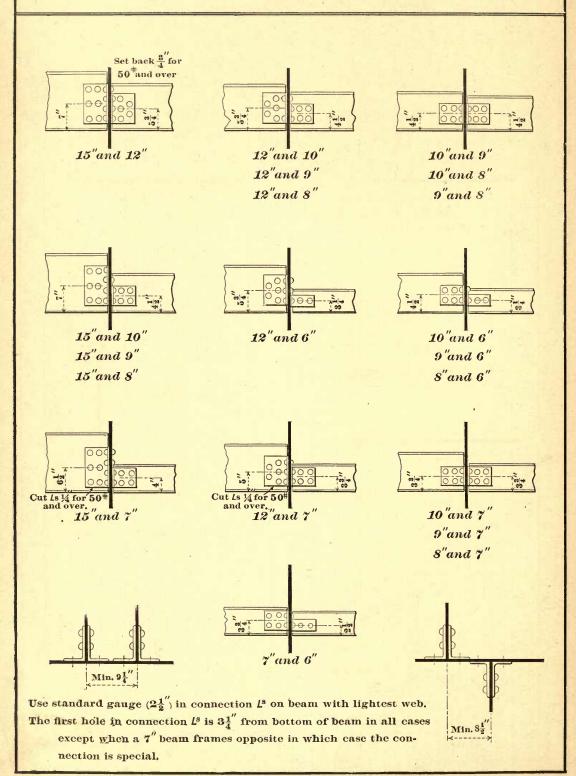
7.2

6.0 215 9 20.5

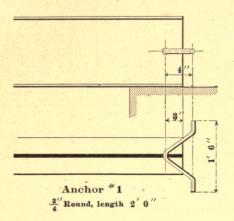
LIGHT

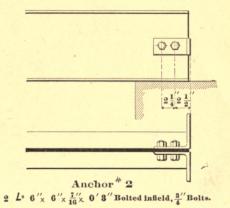
BEAMS

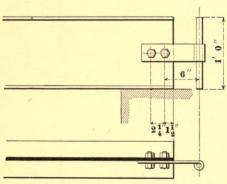
Connections for beams of different depths. (framing opposite)



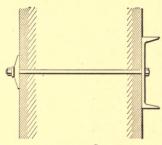
ANCHORS





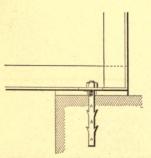


Anchor * 3 $\frac{3''}{4}$ Round, length $\frac{1}{4}$ 0 '' Bolted infield, $\frac{3}{4}$ '' Bolts, $\frac{3}{8}$ '' Flat '' 1''

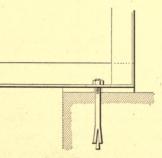


Anchor #4

2"Bolt. Plain, square washer or Cast Iron Rosette.

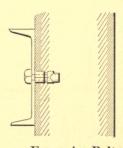


Hacked Bolt,
Size and length of bolts variable,



Split Bolt.

Punch holes $\frac{3}{8}''$ larger than size of bolts.

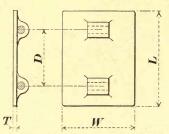


Expansion Bolt.

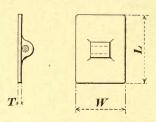
In ordering, give Metal to be fastened, also Diam. and length of Bolt.

SEPARATORS.

All Dimensions in Inches.



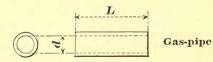




SIZE	ST	ANDARD	DIMENSIO	NS		WEIG	нтѕ		SIZE
OF BEAM	DISTANCE BETWEEN HOLES D	MIN. WIDTH OF SEPARATOR	LENGTH OF SEPARATOR L	THICKNESS $oldsymbol{T}$	SEPARATOR	OF SEPARATOR FOR 1 ADDIT'L SPREAD OF I	BOLTS AND NUTS	OF BOLTS FOR 1" ADDIT'L SPREAD OF I	OF BEAM
24	12	63/4	20	5 8	28.00	4.50	2.84	.248	24
20	12	6	16	66	23.00	3.20	2.70	"	20
18	9	5 3/4	14	66	21.00	2.75	2.60	"	18
15	$7\frac{1}{2}$	5 1/2	$11\frac{1}{2}$	1/2	14.75	1.80	2.40	"	15
12	5	5	834	u	9.75	1.50	2.28	44	12
10	One Hole	$4^{\frac{3}{4}}$	$7\frac{1}{2}$		6.50	1.25	1.08	.124	10
9		$4\frac{1}{4}$	$6\frac{1}{2}$	"	5.75	1.10	1.04	**	9
8		4	$5\frac{1}{2}$		4.50	1.00	1.01		8
7		$3\frac{1}{2}$	5		3.75	.75	0.95		7
6	66	31/4	$4\frac{1}{2}$	ıı	2.25	.60	0.93	i.	6

Bolts $\frac{3}{4}$ diam.

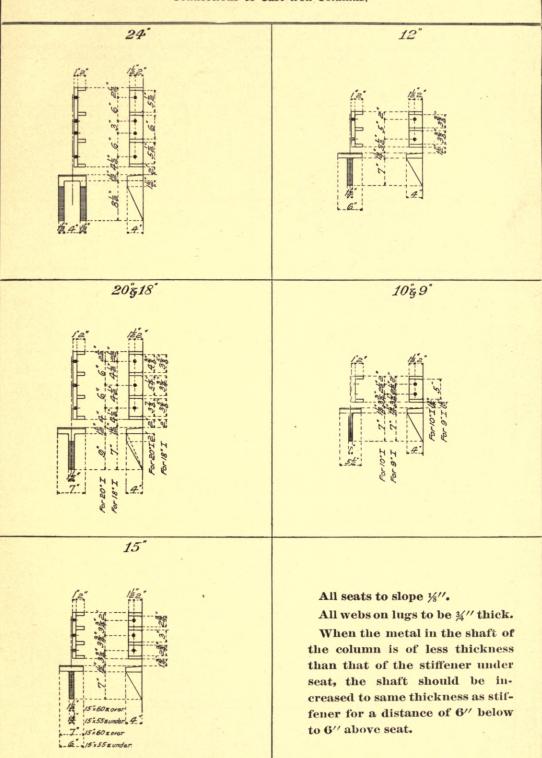
Beams should be spread so that width of separator "W" comes in even quarters of an inch.



	to any end of								
SIZE	S	TANDARD I	DIMENSIO	NS		WEIG	HTS		SIZE
OF BEAM		MIN, LENGTH OF SEPARATOR L	NOMINAL DIAMETER OF PIPE		SEPARATOR	OF SEPARATOR FOR 1" ADDIT'L SPREAD OF I	BOLTS AND NUTS	OF BOLT FOR 1" ADDIT'L SPREAD OF I	OF BEAM
5		3	3-4		.28	.1	.9	.124	5
4		23/4	46		.26	66	.87	"	4
3		$2\frac{1}{4}$.21		.82	ıı	3

Bolts $\frac{3''}{4}$ diam.

BEAMS.
Connections to Cast-iron Columns.



BEAMS AND CHANNELS. Standard Punching in Web.

		MIN.	MIN.					
SIZE OF BEAM	WEIGHT	DIST.	DIST.					
INCHES	LBS.	INCHES	INCHES					
7/	100.0	$2\frac{1}{2}$	31/4	. 24"				
	95.0	0.	"				vn in e	
24	90.0	"	61			-	be pur ration	
	85.0	**	"	21" 21" 21"			nd any	
	80.0	11	61	(co) 35'+				omitt-
	100.0	234	31/2		ed.	3 hole	s 3 ["] apa	rt and
	95.0		.,					cannot
	90.0		**				d centi	ral in
20	85.0		- 11		bear			
	80.0	25/8	38				e 3"plu	
oe, i	75.0	03	01				ven in	
31	70.0	23/8	31/8	18"& 20" 2\"2\"				will in
	65.0	.,	"		1		e be 3	
	90.0				amo	unt gi	ven in	table.
	85.0						lie-Ro	
	80.0 75.0	0.5	33	6			spaced	
18	70.0	2 8	J ₈		Bear		4 to 1	4
1	65,0	11.		8 1 1	Bear	us.		
	60.0	238	318					
	55.0	<u>⊿8</u>	08					
	80.0	$2\frac{5}{8}$	338	15" o'" o'"o'"	MIN. DIST.	MIN. DIST.	WEIGHT	SIZE OF
EL 3	75.0	a	"	2½" 2½" 2½" 12½" 12½"	INCHES	INCHES	PER FT.	CHANNEL INCHES
PA	70.0	0			338	25	55.0	
10-	65.0	$2\frac{1}{2}$	31/4		**	u	50.0	
15	60.0	66	- 11		3	$2\frac{i}{4}$	45.0	
	55.0	23/8	31/8			"	40.0	15
	50.0	**	46				35.0	100
	45.0	$2\frac{1}{8}$	27/8		"	11	33.0	- 3
	42.0	- 11						
	65.0			. 12"		1		
	60.0			$2\frac{1}{4}''2\frac{1}{4}''$				
	55.0				7 12			
	50.0	28				0.3	40.0	
12	45.0	$2\frac{1}{4}$				23/8	40.0 35.0	12
	40.0			63. 18		178		
	35.0	2				18	30.0 25.0	
	31.5					- 11	20.5	
						**	⊿∪,	
1111				9		-		

BEAMS AND CHANNELS.

Standard Punching in Web.

					-	
SIZE OF	WEIGHT	MIN.		MIN. DIST.	WEIGHT	SIZE OF CHAN-
	PER FT.	DIST.		a	PER FT.	NEL
INCHES	LBS.	INCHES		INCHES	LBS.	INCHES
1			10	2	35.0	
	40.0	21/8		- "	30.0	
10	35.0	"		"	25.0	10
	30.0	17/8	- C5 - E8 - E	17/8	20.0	
177	25.0	44		-6	15.0	
	35.0	21/8	21/1 22/1	17/8	25.0	3.5
	30.0	4	$\left \frac{4\frac{1}{2}}{4} \right $	18	20.0	
9			(A		THE STATE OF	9
	25.0	1 7 8	9", 8" & 7"	$1\frac{3}{4}$	15.0	
	21.0	**		44	13.25	-
				17/8	21.25	
5	25.5	2	,,,	"	18.75	
8	23.0	13/4	21" 24 21" k4	"	16.25	8
	20.5			13/4	13.75	
	18.0	"		*	11.25	
				13/4	19.75	
					17.25	
7	20.0	13/4		44	14.75	
	17.5			15/8	12.25	175
	15.0	"		46	9.75	
-	10.0		n n n n			
			6, 5, 4 & 3"	1 5 8	15.50	100
6	17.25			"	13.00	0
	14.75	"		- 44	10.50	
	12.25			**	8.00	
-	14.75	1-1/2		1 5 8	11.50	
5	12.25	"	91/1 24 16 - 41		9.00	5
-	9.75			44	6.50	
	10.5	1 1 1 2	++ =;	$1\frac{1}{2}$		
	9.5			**	7.25	
4	8.5			14	6.25	4
	7.5				5.25	
	7.5	11/2		$1\frac{1}{2}$	6.0	
3		13				3
10	6.5			**	5.0	0
-	5.5	".		**	4.0	

Holes shown in each group can be punched in one operation (at Pencoyd) and any desired hole can be omitted.

Holes for tie-rods should be spaced $2\frac{1}{4}''$ apart for 10'' to 3'' beams.

ANGLES

						,	Weig	ht in	pound	ls per	foot.							
	SIZE	1/8	$\frac{3}{16}$	1/4	<u>5</u>	38	7	1/2	9 16	5/8	11/16	$\frac{3}{4}$	<u>13</u>	7 8	15 16	1	SIZE	
	s ×s							26.4	29.8	33.2	36.6	39.0	42.4	45.8	49.3	52.8	8 ×8	
	s ×s					14.8	17.3	20.0		24.9	26.5	_	31.7	34.1	36.5		c ×c	1
*	5 ×5					12.3	14.3	16.6	18.2	20.2	22.2	24.6	26.7	28.6	30.7		5 ×5	*
	4 ×4				8.2	9.8	11.3	12.8	14.5	16.0	17.7	19.5			l lan		4. ×4	
	$3\frac{1}{2}\times3\frac{1}{2}$				7.1	8.5	9.9	11.1	12.5	13.9				1.5	-		$3\frac{1}{2} \times 3\frac{1}{2}$	
	3 ×3			4.9	6.1	7.3	8.5	9.9.	11.2	12.4							3 3	
*	$2\frac{3}{4} \times 2\frac{3}{4}$			4.5	5.5	6.6	7.7	8.6									$2\frac{3}{4} \times 2\frac{3}{4}$	*
	$2\frac{1}{2} \times 2\frac{1}{2}$		3.1	4.1	5.0	5.9	6.9	7.8									$2\frac{1}{2} \times 2\frac{1}{2}$	
*	$2\frac{1}{4} \times 2\frac{1}{4}$		2.7	3.6	4.5	5.4		11									$2\frac{1}{4} \times 2\frac{1}{4}$	*
	2 ×2		2.5	3.2	4.0	4.8											2 2	
	$1\frac{3}{4} \times 1\frac{3}{4}$		2.1	2.8	3,5	4.1											$1\frac{3}{4} \times 1\frac{3}{4}$	
	$1\frac{1}{2} \times 1\frac{1}{2}$	1.2	1.8	2.4	2.9	3.5					1					E	$1\frac{1}{2}$ X $1\frac{1}{2}$	
1											7							
						V												
																33		-
																	TW Y	
-	SIZE	1/8	3 16	1/4	3 16	3/8	16	1/2	16	<u>5</u>	11 16	$\frac{3}{4}$	13	7/8	15 16	1	SIZE	
*	8 × 6							23.0	25.8	28.8	31.7	34.6	37.6	40.6	43.6	46.7	8 ×6	*
*	7 ×3 1/2							17.0	19.2	21.3	23.5	24.8	27.1	29.2	31.4	34.0	$7 \times 3\frac{1}{2}$	*
	6 ×4					12.2	14.3	16.5	18.1	20.2	22.2	24.4	26.4	28.6	305		6 4	
	6 ×3 ½					11.6	13.5	15.6	17.1	19.0	20.9	23.0	25.0	27.0	29.0		$6 \times 3\frac{1}{2}$	
*	5 ×4					11.0	12.8	14.8	16.2	17.9	19.8	21.9					5 ×4	*
	5 ×3 ½				8.7	10.4		136	15.4	16.9	18.9	20.9					5 ×3 ½	
1	5 ×3				8.2	9.8		⊢	14:4	16.0	17.6	19.5					5 ×3	
	$4 \times 3\frac{1}{2}$				7.7	9.1	10.6	11.9	13.4	15.0	16.5	18.2					$4 \times 3\frac{1}{2}$	
à	4 ×3			1	7.1	8.5	9.9	11.1	12.5	13.9		:					4 ×3	
3	$3\frac{1}{2}\times3$				6.6	7.9	9.2	10.7	12.1	13.4						П	$3\frac{1}{2}\times3$	*
*	$3\frac{1}{2} \times 2\frac{1}{2}$			4.9	6.1	7.2	8.3										$3\frac{1}{2} \times 2\frac{1}{2}$	ľ
	3 ×2 1/2			4.5	5.5	6.6	7.7	8.7									3 ×2 1/2	
	3 ×2		0.77	4.1	5.0	5.9	6.9	1		. *							3 ×2 2 ½ ×2	
	$2\frac{1}{2} \times 2$		2.7	3.6	4.5	54	6.2	7.0									.1	
	$2 \times 1\frac{1}{2}$ $2 \times 1\frac{1}{4}$		1.9	2.9	3.6	4.3						92					$\begin{array}{ccc} 2 & \times 1\frac{\pi}{2} \\ 2 & \times 1\frac{1}{4} \end{array}$	
	~ ^14	12	1.0	2.0	0.0	3.0											-4	
							1				1	1						1
	SIZE	1/8	3 16	1/4	<u>5</u>	3/8	7 16	1/2	9 16	8	11/16	3 4	13	7/8	15	1	SIZE	

ANGLES

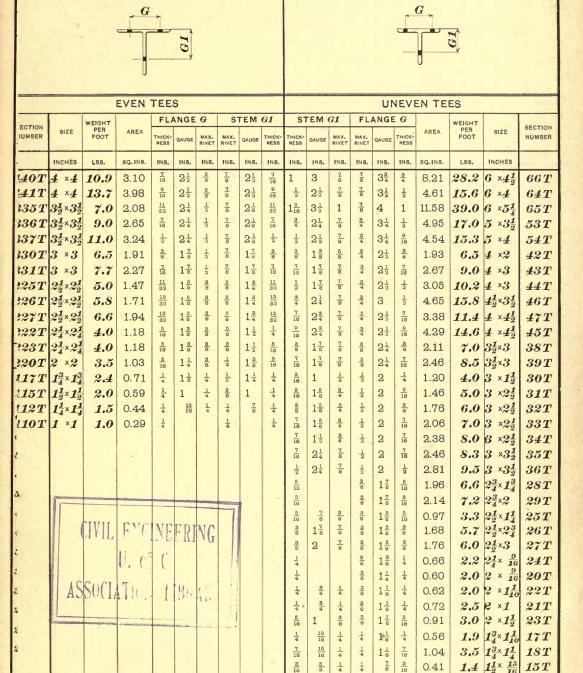
		1					1	letus	al size	of leg	s							
	SIZE	1/8	3 16	1/4	<u>5</u>	3/8	$\frac{7}{16}$	1/2	9 16	5/8	<u>11</u> 16	$\frac{3}{4}$	13 16	7/8	1 <u>5</u>	1	SIZE	9
	8 x 8							8	8 1/16	8 ½	8 3 18	8	8 1 10	8 1/8	8 3 16	81/4	8 ×8	
	6 × 6					6	6 1 16	6 ½	618	61/4	6	6 16	6 ¹ / ₈	6 16	6 4		6 × 6	
*	5 × 5			3		5	5 16	5 1 8	5	5 16	5 1 8	5 18	$5\frac{1}{4}$	5 18	5 3 8		5 ×5	*
	4 × 4				4	416	4 1 8	4	4 16	4 1/8	4 3 18	41/4					4 x4	
d	$3\frac{1}{2} \times 3\frac{1}{2}$				3 1/2	3 16	3 8	$3^{\frac{1}{2}}$	3 18	3 8							$3\frac{1}{2} \times 3\frac{1}{2}$	
i	3 × 3			3	3	310	3 18	3 18	34	3 16							3 ×3	
*	$2\frac{3}{4} \times 2\frac{3}{4}$			2 3 4	2 13 16	$2\frac{7}{6}$	2 16	3									$2\frac{3}{4} \times 2\frac{3}{4}$	*
	$2\frac{J}{2}\times 2\frac{1}{2}$		$2^{\frac{1}{2}}$	2 16	2 8	$2\frac{11}{16}$	2 3 4	2 18									$2\frac{1}{2}\times 2\frac{1}{2}$	
*	$2\frac{1}{4}$ \times $2\frac{1}{4}$		21/4	2 16	$2\frac{3}{8}$	$2\frac{7}{16}$											21 x 21	*
	2 × 2		2	21/16	2 1/8	2 1 R								14			2 x2	
	$1\frac{3}{4} \times 1\frac{3}{4}$		13/4	1 13	1 7 8	1 18											$1\frac{3}{4} \times 1\frac{3}{4}$	
	$1\frac{1}{2} \times 1\frac{1}{2}$	$1\frac{1}{2}$	1 16	1 5	1 11	1 3 4											$1\frac{1}{2} \times 1\frac{1}{2}$	
																0		
			3															
	SIZE	1/8	3 16	1/4	<u>5</u>	$\frac{3}{8}$	7/16	1/2	9 16	<u>5</u>	11 16	$\frac{3}{4}$	13 16	78	$\frac{15}{16}$	1	SIZE	
*	8 ×6							6	610	6 1/8	6 3	6 4	6 16	6 3 8	6 7 16	61/2	S ×6	*
*	$7 \times 3\frac{1}{2}$							3 1/2	3 16	3 8	3 11/16	3 2	3 16	3 8	3 11	3 4	$7 \times 3\frac{1}{2}$	*
	6 × 4					4	4 16	41/8	4	4 16	4 1/8	4 3 16	4 1/4	4 18	43		6 ×4	
	$G \times 3\frac{1}{2}$					$3^{\frac{1}{2}}$	3 18	3 8	$3^{\frac{1}{2}}$	3 18	3 6	3 11/16	3 4	3 18	3 7 8		$6 \times 3\frac{1}{2}$	
*	5 x4					4	4 16	418	4	4 18	4 1/8	4 3 16					5 ×4	*
	5 x 3 2		F .		3 1/2	318	3 8	$3^{\frac{1}{2}}$	3 16	3 8	3 10	3 4					5 × 3 ½	
	5 ×3				3	31/16	3 8	3	3 16	3 1/8	3 16	3 4					5 ×3	
	$4 \times 3\frac{1}{2}$				3 1/2	3 16	3 6	$3^{\frac{1}{2}}$	3 16	3 8	3 11	3 4					$4 \times 3\frac{1}{2}$	
	4 × 3				3	316	3 1/6	3	3 16	3 1/8							4 x3	
	3 1/2 x 3				3	3 16	3 1 8	3 16	31/4	3 18						7.2	$3\frac{1}{2} \times 3$	
*	3 1/2 × 2 1/2			$2^{\frac{1}{2}}$	2 16	2 6 8	211	2 3/4									$3\frac{1}{2} \times 2\frac{1}{2}$	*
	3 × 2 ½			$2^{\frac{1}{3}}$	2 16	2 8	211	$2\frac{3}{4}$									3 × 2 ½	
	3 × 2			2	2 18	21/8	$2\frac{3}{16}$	$2^{\frac{1}{4}}$									3 ×2	
	$2\frac{1}{2} \times 2$		2	2:8	2 1/8	2 3 16	$2^{\frac{1}{4}}$	$2\frac{5}{16}$								T .	9 1 × 2	
	$2 \times 1\frac{1}{2}$		11/2	1 10	1 5 6	1 16											$\frac{2}{2}$ × $1\frac{1}{2}$	
	2 ×11/4		11/4	1 1 1 1 1 1 1 1	1 3 8	1 7 16	Te										2 ×1 4	
	SIZE	1/8	3 16	1/4	5 16	3/8	7/16	1 2	9 16	5 8	11 16	$\frac{3}{4}$	13 16	7/8	15 16	1	SIZE	
ii	Note			tes fir						s mar					16		4	
		_											_					1

ANGLES

Area		~~	0 10	ina	hog
Area	111	Suu	utre	HILL	LLCS.

SIZE \frac{I}{S} \frac{3}{16} \frac{I}{I} \frac{7}{16} \frac{7}{3} 7	ŀ																		
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		SIZE	1/8	3 16	1/4	5 16	3/8	7	1/2	9 16	5/8	11 16	3 4	13	7/8	15	1	SIZE	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		8 ×8							7.76	8.76	9.76	10.76	11.47	12.47	13.47	14.50	15.53	8 x 8	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		6 × 6				-	4.35	5.09	5.88	6.60	7.32	7.79	8.60	9.32	10.03	10.73		G xG	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4	5 x 5					3.62	4.21	4.89	5.35	5.94	6.5.3	7.24	7.86	8.41	9.03		5 × 5	*
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	4 ×4				2.41	2.88	3.32	3.76	4.26	4.70	5.20	5.73				- ,	4 ×4	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	$3\frac{1}{2} \times 3\frac{1}{2}$				2.09	2.50	2.91	3.26	3.68	4.09				- :			$3\frac{1}{2} \times 3\frac{1}{2}$	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	l	3 ×3			1.44	1.79	2.16	2.50	2.90	3.28	3.65							3 × 3	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		$2\frac{3}{4} \times 2\frac{3}{4}$			1.32	1.65	1.94	2.26	2.53								V 5	$2\frac{3}{4}$ x $2\frac{3}{4}$	*
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	$2\frac{1}{2}$ x $2\frac{1}{2}$		0.91	1.21	1.47	1.74	2.03	2.29	8								$2\frac{1}{2} \times 2\frac{1}{2}$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	-		0.79	1.06	1.32	1.59											$2\frac{1}{4}$ $\times 2\frac{1}{4}$	*//*
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	E			0.74	0.94	1.18	1.41						L					100	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	н				- 1	'	-											$1\frac{3}{4} \times 1\frac{3}{4}$	
SIZE $\frac{1}{8}$ $\frac{3}{16}$ $\frac{7}{4}$ $\frac{5}{16}$ $\frac{3}{16}$ $\frac{3}{8}$ $\frac{7}{16}$ $\frac{1}{2}$ $\frac{9}{16}$ $\frac{5}{8}$ $\frac{11}{16}$ $\frac{3}{4}$ $\frac{28}{16}$ $\frac{7}{8}$ $\frac{15}{16}$ 1 SIZE 8×6 6.76 7.59 8.47 9.32 10.17 11.06 11.94 12.82 13.73 8×6 $7 \times 3\frac{1}{2}$ 6×4 $8.3\frac{1}{2}$ 8×6	ı	-	0.35				1.03												
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı							~										10	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı										1		(2)						
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı								=										
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı																		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		SIZE	1/8	3 16	1/4	<u>5</u>	3/8	7 16	1/2	9 16	<u>5</u>	11 16	34	13	7 8	15 16	1	SIZE	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		8 x6							6.76	7.59	8 47	9.32	10.17	11.06	11.94	12.82	13.73	8 x 6	~
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1	7 x3 1/2		4.					5.00	5.65	6.27	6.91	7.30	7.97	8.60	9.23	10.00	$7 \times 3\frac{1}{2}$	1
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	۱	6 x4					3.60	4.21	4.85	5.32	5.94	6.53	7.18	7.76	8.41	8.97		6 x4	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	и	~	9	701	1		3.41	3.97	4.60	5.03	5.59	6.15	6.76	7.35	7.94	8.53		$6 \times 3\frac{1}{2}$	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	П	CONT.					3.24	3.76	4.35	4.76	5.26	5.82	6.44					1000	7
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	~																~	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı										-								
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı	~			- 1		1					4,85	5.35						
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	ı										100					31			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					1 44						3.94				G				7
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$											П						100	The second second	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$																			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$				0.79		1									-			1	
SIZE $\frac{1}{8}$ $\frac{3}{16}$ $\frac{1}{4}$ $\frac{5}{16}$ $\frac{3}{8}$ $\frac{?}{16}$ $\frac{1}{2}$ $\frac{9}{16}$ $\frac{5}{8}$ $\frac{11}{16}$ $\frac{3}{4}$ $\frac{13}{16}$ $\frac{?}{8}$ $\frac{15}{16}$ 1 SIZE																			
		$2 \times 1\frac{1}{4}$		0,56	0.76	0.97	1.15			11 [HE.	:=12				2 x1\frac{1}{4}	
		SIZE	1/8	3 16	1/4	5 16	3 8	7 16	1/2	9 16	5 8	11 16	3 4	13 16	7' 8		1	SIZE	

TEES.
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Weights, Areas, Dimensions, etc., etc.



NOTE: In ordering Tees give size, weight and section number.
In giving size of Uneven Tees flange should be given first.

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0.35

1.2 11× 15

12T

Z- BARS
Weights, areas, dimensions, etc., etc.



¥ F001 S S S F	NO	MINAL	SSE	ACTUAL SIZE	WEIGHT		GAUGE	MAX. F	RIVETS	GAUGE	SSH	NOMINAL
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	S	SIZE	THICK	FLANGES & WEB		AREA	G	G	G1	G1	THICK	SIZE
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	1N	CHES	INCHES	INCHES	POUNDS	SQ-INCHES	INCHES	INCHES	INCHES	INCHES	INCHE8	INCHES
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		3		$\begin{array}{cccccccccccccccccccccccccccccccccccc$	8.29 10.00 11.15	2.44 2.94 3.25 3.75	15 18	11	 	"	5 16 3 8 7 16	3
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	•	4	1 3 10 3 8 7 10 10 10 5 8 10 10 5 8 10 10 10 10 10 10 10 10 10 10	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	9.89 11.90 13.46 15.50 17.54 18.80 20.87	2.91 3.52 3.96 4.56 5.16 5.55 6.14	 178 2		11 11 11 11 11	" " " " " " "	5 16 3 8 7 16 12 9 16 5 8	4
$ 6 \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$		5	3 8 7 16 1 2 9 16 5 8 11 16	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	13.77 16.15 17.78 20.09 22.44 23.66	4.05 4.75 5.23 5.91 6.60 6.96	 218				38 77 16 12 9 16 58 11 16	5
		6	3 8 16 12 9 16 5 8 11 16 3 4 16 7 8	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	18.32 21.05 22.71 25.36 28.05 29.37 31.89	5.39 6.19 6.68 7.46 8.25 8.64 9.38		0 0 0 0		11 11 11 11	3 8 10 10 12 9 10 10 10 10 10 10 10 10 10 10 10 10 10	6

Z-BAR COLUMNS

Weights, areas,

Y

dimensions, etc., etc.

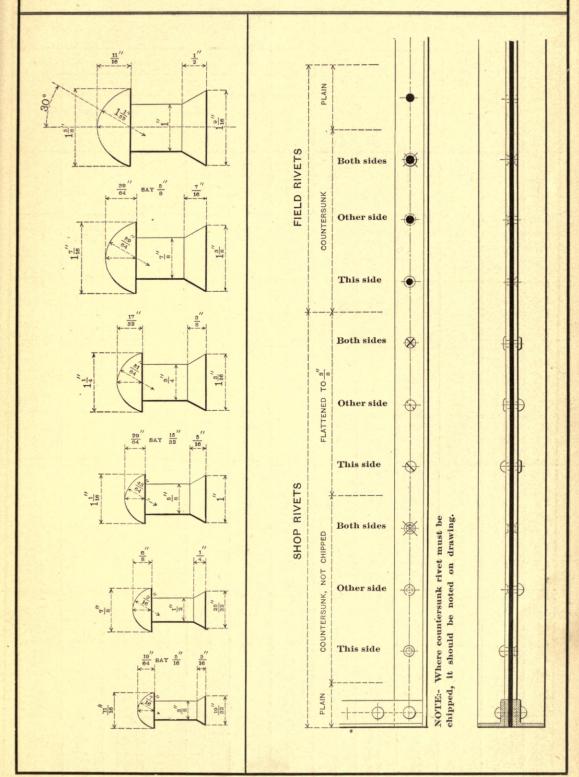
SIZE OF COLUMN	84	SIZE OF Z-BARS	міртн	GAUGE	TANGT	STANDARD DIMENSIONS	AXIS	x-x.	AXIS	Ÿ-Y.	WEIGHT	AREA	BIZE OF COLUMN
1N8.	SIZE	SIZE OF FLANGES	W		t	STANDARD DIMENSIONS	MOMENT OF INERTIA	OF GYRA- TION	OF INERTIA	OF GYRA- TION	PER	SQ.IN.	1N8.
		1 2 × 3 × 2 × 3	1	-	3 8	44//	84.7	3.0	31.7	1,8	31.5	9.26	
	AR	16 21 x 31 x 21		1	7 18	61/4"	105.1		41.8			11.64	
	A8 Z-BAR	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$			5 글 1끊		125.1 134.6		53.4 55.2			14.01 15.63	
6	Web	1 2 3 x 3 1 x 2 3			178	11/2 3" 1%	153.1		67,1			18.00	6
	G" HICKN					11/10 -							
	G" Wel												
	00					g' 814" g		-				- 1	
		1 27 x 4 x 27	81/4		7 ½	13"	134.7		65.7			11.03	
	4	18 215 x 416 x 216	1		73/18	71/2"	166.9		85.8			13.83	
	-BAR	$\frac{3}{8}$ 3 x $4\frac{1}{8}$ x 3	8 5 8 8 8 7 10 10 10 10 10 10 10 10 10 10 10 10 10		7-1/4 3 13/18				107.8 115.6			16.71 18 90	1
IA.	7" Web SAME THICKNESS AS Z-BAR	1			378	11/6" 31/6" 11/6			138.6	1		21.74	
8	7" Web	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	813		3 15 3 18				163.0			24.58	8
	THIC	5 3½ × 4 × 3½			3=	176			167.3			26.58	
	SAME	11 3 x 4 x 3 x 3 x 3 x 3 x 3 x 3 x 3 x 3 x		21 6	3 18		323.8	3,3	192.8	2.5	99.9	29.37	
		$\frac{3}{4}$ $3\frac{3}{16} \times 4\frac{1}{8} \times 3\frac{3}{16}$	9	21/8	3 8	\overrightarrow{g} \overrightarrow{g}	351.5	3.3	220.5	2.6	109.7	32.25	
		$\frac{5}{16}$ $3\frac{3}{16} \times 5 \times 3\frac{3}{16}$	105	111 8	$9\frac{1}{16}$	13"	193.8	3.5	147.4	3.0	53.1	15.63	
		$\frac{3}{8}$ $3\frac{1}{4} \times 5\frac{1}{10} \times 3\frac{1}{4}$	10-1	13/4	1 8	71/11			183.4			18.83	
10	AR	$\frac{7}{16}$ $3\frac{5}{16} \times 5\frac{1}{8} \times 3\frac{5}{16}$	1011	113 8) <u>3</u>		267.6	3.4	222.0	3.1	75.0	22.06	
1	7" Web THICKNESS AS Z-BAR	$\frac{1}{2} 3\frac{1}{4} \times 5 \times 3\frac{1}{4}$	101	17/9 8	3 3	11%" 31%" 11%			234,4			24.42	
10	Web	$\frac{9}{16}$ $3\frac{5}{16} \times 5\frac{1}{16} \times 3\frac{5}{16}$			3 18				273.7			27.58	10
	HICK!	5 3 3 x 5 1 x 3 8			3 %	136"			315.6				
	SAME T	$\frac{11}{16}$ 3 $\frac{1}{4}$ x 5 x 3. $\frac{1}{4}$			37				320.0		111.0		
100	00	$\frac{3}{4}$ $3\frac{5}{16} \times 5\frac{1}{16} \times 3\frac{5}{16}$	10 %	21/8 8	3 1/2		395.5	3.3	363.0	3.1	121.7	35.81	
,						$g \longrightarrow g$							
		$\frac{3}{8} 3\frac{1}{2} \times 6 \times 3\frac{1}{2}$	193	1 3 11		a.//	337.0	30	287.8	36	72.6	21.36	
		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				14" 8¾"			346.9				
		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$						0.0	409.2				
	BAR.	$\begin{array}{cccccccccccccccccccccccccccccccccccc$					1		426.3				
	A8 Z	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				x 278 x 278			489.2				
12	" WE	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				A CONSCIONAL CONSCIONA			555.8		1		12
	84WE THICKNESS AS Z-BAR	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		1 1		13/1			562.4			1	
- 0	SAME	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$					l 1		628.2				
		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				Y. TY			699.1		1		
		8 08 08 08	200		4	[g] g	300.0						
						RIVETS 4 DIAM.							

SHEARING AND BEARING VALUE OF RIVETS.

BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT \$2000, PER SQ. INCH. 1																	
### 100 10											_				•		
3 .375 .1104 600 1190 1410 1600 1800 2250 2800 3000	DIAM.	OF RIVET		SHEAR		BEARIN	NG VALU	E FOR D	IFFERE	NT THIC	KNESS C	F PLATE	E AT 120	000 [#] P	ER SQ. I	NCH.	
1		DEC'L	SQ. INS.	GÔUÕ	1/4	<u>5</u>	3 8	7 16	1 2	9 16	5 8	11/16	3/4	1 <u>3</u> 16	7/8	15 16	1
\$\frac{8}{3} \cdot 3.006	38	.375	.1104	660	1130	1410	1690						1				
3	12	.500	.1963	.1180	1500	1880	2250	2630	3000							n Bal	
T	5	.625	.3068	1840	1880	2340	2810	3280	3750	4220	4690						
1 1,000 1,7864 4710 3000 3750 4500 3250 6000 6750 7500 8250 9000 9750 10500 11250 12000		.750	.4418	2650	2250	2810	3360	3940	4500	5160	5630	6190	6750				
DIAM. OF RIVIT. AREA BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 13090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 14090, PER SQ. INCH. BEAR SQ. INCH.	3	.875	.0013	3610	2630	3280	3940	4590	5250	5910	6560	7220	7880	8530	9190	9840	
FRAC DEC'L 90. NS. 7300	1	1.000	.7854	4710	3000	3750	4500	5250	6000	6750	7500	8250	9000	9750	10500	11250	12000
FRAC. DEC'L 90. NS. 7300 1 4 5 8 8 7 1 1 2 9 6 5 11 3 4 18 7 15 16 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	OJAM.	OF RIVLT		SHEAR		BEARIN	NG VALU	E FOR D	FFERE	NT THIC	KNESS (F PLATI	E A,T 15	000, [#] P	ER SQ. I	NCH.	
S 1.00	FRAC.	DEC'L		7500	1/4	<u>5</u>	38	7 16	$\frac{1}{2}$	9 16	58	11/16	34	13 16	7 8	15 16	1
S	38	.375	.1104	830	1410	.1760	2110										
3		.500	.1963	1470	1880	2340	2810	3280	3750								
Total Color Colo	58	,625	.3068	2300	2340	2930	3520	4100	4690	5280	5860						
1 1,000 1,7854 5890 3750 4690 5820 8580 7500 8440 9380 10310 11250 12190 13130 14060 15000	$\frac{3}{4}$.750	,4418	3310	2810	3520	4220	4920	5630	6330	7030	7720	8440				
CIAMA, OF RIVET AREA BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 22000, PER SQ. INCH. BEAC. DEC'L SQ. INS. 1000 1	3	.875	. 6013	4510	3280	4100	4920	5740	6560	7380	8200	9030	9850	10670	.11480	12300	
FRAC. DEC'L SQ. INS. 11000 1 4 15 8 7 16 1 2 9 16 8 11 3 16 7 8 15 16 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1	1.000	.7854	5890	3750	4690	5620	6560	7500	8440	9380	10310	11250	12190	13130	14060	15000
1 1 1 1 1 1 1 1 1 1	DIAM.	OF RIVET		SHEAR		BEARIN	NG VALU	E FOR D	IFFERE	NT THIC	KNESS (OF PLAT	E AT 22	000, [#] P	ER SQ.	INCH.	
1	FRAC.	DEC'L	SQ. INS.	11000	1/4	<u>5</u> 16	3 8	7 16	1/2	<u>9</u> 16	58	<u>11</u> 16	3 4	<u>13</u> 16	7 8	15 16	1
5 / 8 .625 .3068 3370 3440 4300 5160 6020 6880 7740 8600 1 2380 2370 2418 4860 4130 5160 6190 7220 8250 9280 10320 11340 12380 12380 12380 12340 14440 15640 16840 18050 <th< th=""><th>38</th><th>.375</th><th>.1104</th><th>1210</th><th>2060</th><th>2580</th><th>3090</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th>141</th><th></th></th<>	38	.375	.1104	1210	2060	2580	3090									141	
3	12	.500	1963	2160	2750	3440	4130	4820	5500	18.7							
7 8 .875 .6013 6610 4810 6020 7220 8430 9630 10840 12040 13240 14440 15640 18840 18050 1 1.000 .7854 8640 5500 6890 8250 9630 11000 12380 13750 15130 16500 17880 19250 20630 22000 DIAM. OF RIVET AREA IN AT # 2000 1 4 5 16 3 7 18 1 2 9 18 5 11 16 3 18 18 7 8 16 18 18 18 18 18 18 18 18 18 18 18 18 18	58	.625	.3068	3370	3440	4300	5160	6020	6880	7740	8600						
1 1.000 .7854 8640 5500 6880 8250 9630 11000 12380 13750 15130 16500 17680 19250 20630 22000 10140 .0 F RIVET AREA IN AT # BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 24000, PER SQ. INCH. RAC. DEC'L SQ. INS. 12000 1/4 1/5 1/8 3/8 1/7 1/8	$\frac{3}{4}$.750	.4418	4860	4130	5160	6190	7220	8250	9280	10320	11340	12380				
DIAM. OF RIVET AREA IN SHALE SHEAR AT # BEARING VALUE FOR DIFFERENT THICKNESS OF PLATE AT 24000, PER SQ. INCH. FRAC. DEC'IL SQ. INS. 12000 1 4 5 16 3 7 16 1 2 9 16 5 11 3 13 7 8 15 16 1 3 .375 .1104 1320 2250 2810 3380	78	.875	. 6013	6610	4810	6020	7220	8430	9630	10840	12040	13240	14440	15640	16840	18050	
TRAC. DEC'L SQ. INS. 12000 $\frac{1}{4}$ $\frac{5}{16}$ $\frac{3}{8}$ $\frac{7}{16}$ $\frac{1}{2}$ $\frac{9}{16}$ $\frac{5}{8}$ $\frac{1!}{16}$ $\frac{3}{4}$ $\frac{13}{16}$ $\frac{7}{8}$ $\frac{15}{16}$ 1 $\frac{3}{8}$.500 .1963 2360 3000 3750 4500 5250 8000 $\frac{5}{8}$.625 .3068 3680 3750 4690 5620 6560 7500 8440 9370 $\frac{3}{4}$.750 .4418 5300 4500 5620 6750 7670 9000 10120 11250 12370 13500 $\frac{7}{8}$.875 .6013 7220 5250 6560 7870 9190 10500 11810 13120 14440 15750 17060 18370	1	1.000	.7854	8640	5500	6880	8250	9630	11000	12380	13750	15130	16500	17880	19250	20630	22000
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	DIAM.	OF RIVET		SHEAR SHEAR													
\$\frac{1}{3}\$.500 .1963 2360 3000 3750 4500 5250 6000 \$\frac{5}{8}\$.625 .3088 3680 3750 4690 5620 6560 7500 8440 9370 \$\frac{3}{4}\$.750 .4418 5300 4500 5620 6750 7870 9000 10120 11250 12370 13500 \$\frac{7}{3}\$.875 .6013 7220 5250 6560 7870 9190 10500 11810 13120 14440 15750 17060 18370	-RAC.	DEC'L		12000	1/4	<u>5</u> 16	3 8	7 16	1/2	<u>9</u> 16	5 8	11 16	3 4	13 16	78	15 16	1
\(\frac{5}{8} \) .625 .3068 3680 3750 4690 5620 6560 7500 8440 9370 \(\)	8	-375	.1104	1320	2250	2810	3380										
3/4 .750 .4418 5300 4500 5620 6750 7870 9000 10120 11250 12370 13500 7/8 .875 .8013 7220 5250 6560 7870 9190 10500 11810 13120 14440 15750 17060 18370	1/2	.500	.1963	2360	3000	3750	4500	5250	6000								
7/8 .875 .6013 7220 5250 6560 7870 9190 10500 11810 13120 14440 15750 17060 18370		.625	.3068	3680	3750	4690	5620	6560	7500	8440	9370						
		.750	.4418	5300	4500	5620	6750	7870	9000	10120	11250	12370	13500	:			
1 1,000 7654 9430 8000 7500 9000 10500 13500 15000 16500 18000 19500 21000 22500 24000	7/8	.875	. 6013	7220	5.250	6560	7870	9190	10500	11810	13120	14440	15750	17060	18370		
	1	1.000	.7854	9430	6000	7500	9000	10500	12000	13500	15000	16500	18000	19500	21000	22500	24000

RIVETS.

Proportions and Conventional Signs.



RIVETS

Lengths of fieldrivets for variant grips.

Dimensions in Inches

L						Dime	nsions ln I	nches					
	GRIP			Grip Length DIAMETER			GRIP	1		Grip Length DIAMETER			GRIP
L		1/2	8	$\frac{3}{4}$	7/8	1		1/2	8	34	7/8	1	
_	1 2 5 8 3 4 7 8	$ \begin{array}{c} 1\frac{5}{8} \\ 1\frac{3}{4} \\ 1\frac{7}{8} \\ 2 \\ 2\frac{1}{8} \end{array} $	$ \begin{array}{c} 1^{\frac{7}{8}} \\ 2 \\ 2^{\frac{1}{8}} \\ 2^{\frac{1}{4}} \\ 2^{\frac{3}{8}} \end{array} $	2 2½ 2½ 2¼ 2¾ 2½	$\begin{array}{c} 2\frac{1}{8} \\ 2\frac{1}{4} \\ 2\frac{3}{8} \\ 2\frac{1}{2} \\ \end{array}$	2 2 2 5 le 2 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	12 5 8 3 4 7 8	$ \begin{array}{c} 1\frac{1}{4} \\ 1\frac{3}{8} \\ 1\frac{1}{2} \\ 1\frac{5}{8} \\ 1\frac{3}{4} \end{array} $	$ \begin{array}{c c} 1\frac{3}{8} \\ 1\frac{1}{2} \\ 1\frac{5}{8} \\ 1\frac{3}{4} \\ 1\frac{7}{8} \end{array} $	1 ³ / ₈ 1 ¹ / ₂ 1 ⁵ / ₈ 1 ³ / ₄ 1 ⁷ / ₈	$ \begin{array}{c} 1^{\frac{1}{2}} \\ 1^{\frac{5}{8}} \\ 1^{\frac{3}{4}} \\ 1^{\frac{7}{8}} \end{array} $	$ \begin{array}{c} 1\frac{1}{2} \\ 1\frac{5}{8} \\ 1\frac{3}{4} \\ 1\frac{7}{8} \end{array} $	1 2 5 8 3 4 7 8
	1\frac{1}{8} 1\frac{1}{4} 1\frac{1}{8} 1\frac{1}{2} 1\frac{5}{8} 1\frac{3}{4}	$2\frac{1}{4}$ $2\frac{3}{8}$ $2\frac{1}{2}$ $2\frac{4}{4}$ $2\frac{5}{4}$ $2\frac{7}{8}$ 3	2½ 258 234 3 318 314	$\begin{array}{c} 2^{\frac{5}{8}} \\ 2^{\frac{3}{4}} \\ 2^{\frac{7}{8}} \\ 3^{\frac{1}{8}} \\ 3^{\frac{1}{4}} \\ 3^{\frac{3}{8}} \end{array}$	$\begin{array}{c} 2\frac{3}{4} \\ 2\frac{7}{8} \\ 3 \\ 3\frac{1}{4} \\ 3\frac{3}{8} \\ 3\frac{1}{2} \end{array}$	2 3 3 3 5 8 5 8 5 8 5 8 5 8 5 8 5 8 5 8 5	1 1 8 1 1 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1	$1\frac{7}{8}$ 2 $2\frac{1}{8}$ $2\frac{1}{4}$ $2\frac{3}{8}$ $2\frac{1}{2}$	$\begin{array}{c} 2 \\ 2\frac{1}{8} \\ 2\frac{1}{4} \\ 2\frac{3}{8} \\ 2\frac{1}{2} \\ 2\frac{5}{8} \end{array}$	$\begin{array}{c} 2\\ 2^{\frac{1}{18}}\\ 2^{\frac{1}{4}}\\ 2^{\frac{1}{2}}\\ 2^{\frac{5}{18}}\\ 2^{\frac{9}{4}} \end{array}$	$\begin{array}{c} 2\frac{1}{8} \\ 2\frac{1}{4} \\ 2\frac{3}{8} \\ 2\frac{1}{2} \\ 2\frac{5}{8} \\ 2\frac{3}{4} \end{array}$	$2\frac{1}{8}$ $2\frac{1}{4}$ $2\frac{5}{8}$ $2\frac{5}{8}$ $2\frac{3}{4}$ $2\frac{7}{8}$	$1\frac{1}{8}$ $1\frac{1}{4}$ $1\frac{3}{8}$ $1\frac{1}{2}$ $1\frac{5}{8}$ $1\frac{3}{4}$
	178 2 18 2 14 3 8 1 2 2 5 8 3 4 7 8 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3 -14 3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 5 8 3 4 3 7 8 4	3 5 5 8 3 4 4 1 5 4 4 1 5 6 1	3 5 8 3 4 3 7 8 4 1 8 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1 4 4 1	3 ⁸ / ₄ 3 ⁷ / ₈ 4 4 ¹ / ₄ 4 ¹ / ₄ 4 ¹ / ₈	17/8 2 21/8 21/4 23/8 21/2	2 ^{5/8} 2 ^{9/4} 2 ^{7/8} 3 3 ^{1/8} 3 ^{1/4}	$ \begin{array}{c} 2\frac{3}{4} \\ 2\frac{7}{8} \\ 3 \\ 3\frac{1}{8} \\ 3\frac{1}{4} \\ 3\frac{3}{8} \end{array} $	2 ⁷ / ₈ 3 3 ¹ / ₉ 3 ¹ / ₄ 3 ⁸ / ₈ 3 ¹ / ₁₂	2 ⁷ / ₈ 3 3 ¹ / ₈ 3 ¹ / ₄ 3 ³ / ₈ 3 ¹ / ₄	3 3 ¹ 8 3 ¹ 4 3 ² 8 3 ¹ 5 3 ¹ 8	17/8 2 2 1/8 2 1/4 2 3/8 2 1/2
	3	3 ⁷ / ₈ 4 4 ¹ / ₁₈ 4 ⁵ / ₈ 4 ¹ / ₁₂	4 1/8 4 1/4 4 3/8 4 1/8 4 3/4 4 3/4 4 3/4	4 ³ / ₄ 4 ³ / ₈ 4 ¹ / ₂ 4 ³ / ₄ 4 ⁷ / ₈	4 3 8 4 1 2 4 5 8 4 7 8 5 5	4 ½ 4 ½ 4 ½ 4 ½ 4 ½ 4 ½ 4 ½ 4 ½ 4 ½ 4 ½	2 \frac{5}{8} 2 \frac{3}{4} 2 \frac{7}{8} 3	3 5 8 3 5 8 3 7 8 4	$3\frac{1}{2}$ $3\frac{5}{8}$ $3\frac{3}{4}$ $3\frac{7}{8}$	3 5 8 3 4 3 7 8 4 4 1 8 4 1 8	$3\frac{5}{8}$ $3\frac{5}{4}$ $3\frac{7}{8}$ $4\frac{1}{8}$ $4\frac{1}{4}$	$3\frac{3}{4}$ $3\frac{7}{8}$ 4 $4\frac{1}{4}$ $4\frac{3}{8}$	25/8 23/4 27/8 3
	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	4 5 8 4 4 4 7 8 5 5 1 8 5 1 4 4 5 1 4 5 1	4 4 7 6 5 1 8 5 1 8 5 5 1 8 5 5 1 8 5 5 1 8 5 1	5 5 1 8 5 1 8 5 5 5 5 5 5 5 5 5 5 5 5 5	5 5 5 5 5 5 5 5 5	5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	4 4 1 8 4 1 4 1 8 8 4 1 2 4 5 8 4 3 4 1 4 1 4 1 1 1 1 1 1 1 1 1 1 1 1 1	$\begin{array}{c} 4 \\ 4^{\frac{1}{4}} \\ 4^{\frac{3}{8}} \\ 4^{\frac{1}{2}} \\ 4^{\frac{5}{8}} \\ 4^{\frac{7}{8}} \end{array}$	$4^{\frac{1}{8}}$ $4^{\frac{1}{4}}$ $4^{\frac{3}{8}}$ $4^{\frac{1}{2}}$ $4^{\frac{5}{8}}$ $4^{\frac{5}{8}}$ $4^{\frac{5}{4}}$	4 4 5 8 4 1 2 4 5 8 4 7 8 5 8 5 8 5 8 5 8 6 7 8 6 7 8 6 7 8 6 7 8 7 8 7 8 7 8 7	$4\frac{1}{8}$ $4\frac{1}{2}$ $4\frac{5}{8}$ $4\frac{3}{4}$ $4\frac{7}{8}$ 5 $5\frac{1}{8}$	18 14 38 19 58 34 78 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3
	4 4 1 1 4 3 8 4 1 2 4 5 8 4 3 4 4 4	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	5 8 5 8 5 7 8 6 6 1 1/3 6 6 1 1/3	5 3/4 5 7/8 6 6 1/8 8/8 6 5/8 6 5/8 6 6 5/8	5 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	6 6 1 8 6 1 4 6 8 8 6 8 6 8 6 7 8 6 7 8	4 18 1 48 1 48 1 4 1 1 2 1 8 1 4 1 1 2 1 8 1 1 1 2 1 1 1 1 1 1 1 1 1 1 1	47 47 8 5 5 1 8 5 1 4 4	5 5 5 1 1 4 5 3 8	5 1 8 5 1 4 5 8 8 5 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	5 1 8 5 1 1 2 5 5 8 5 7 8 5 5 7 8	5 5 5 5 5 5 6	4 418 414 438 4258 4258 434
	47/8 5 51/8 51/4 53/8	6 3 6 1 2 6 5 8 6 3 4 6 7 8	$ \begin{array}{c c} 6\frac{5}{8} \\ 6\frac{3}{4} \\ 6\frac{7}{8} \\ 7 \\ 7\frac{1}{8} \end{array} $	$ \begin{array}{c c} 6\frac{3}{4} \\ 6\frac{7}{8} \\ 7 \\ 7\frac{1}{8} \\ 7\frac{1}{4} \end{array} $	$ \begin{array}{c c} 6\frac{7}{8} \\ 7 \\ 7\frac{1}{8} \\ 7\frac{1}{4} \\ 7\frac{3}{8} \end{array} $	$ 7 $ $ 7\frac{1}{8} $ $ 7\frac{1}{4} $ $ 7\frac{3}{8} $ $ 7\frac{1}{2} $	5 5 5 5 5 1 5 3 5 3 5 7 8 5 8				$ \begin{array}{c} 6 \\ 6 \frac{1}{8} \\ 6 \frac{1}{4} \\ 6 \frac{3}{8} \\ 6 \frac{1}{2} \end{array} $	6 1/4 6 8/8 6 1/2 6 8/8 6 8/8 6 1/2 6 8/8 6 1/2 6 8/8 6 1/2	$ \begin{array}{c} 4\frac{7}{8} \\ 5 \\ 5\frac{1}{8} \\ 5\frac{1}{4} \\ 5\frac{3}{8} \end{array} $

BOLTS
Lengths for variant grips



GRIP			IAMETER			GRIP	GRIP			DIAMETER			GRIP
GRIP	1/2	<u>5</u>	3/4	7/8	1	GRIF		1/2	<u>5</u>	34	78	1	
1 2 5 8 3 4 7 8	$1^{\frac{1}{4}}$	11/4	$1\frac{1}{2}$	112	13/4	1 2 5 8 3 4 7 8	$5\frac{1}{2}$	61/4	64	$6^{\frac{1}{2}}$	61/2	634	5 \frac{1}{2} 5 \frac{5}{8}
8	$1\frac{1}{4}$	11/2	11/2	13/4	134	8	$5\frac{5}{8}$	$6\frac{1}{4}$	61	61/2	63	63	$5\frac{5}{8}$
3 4	11/2	11/2	13/4	13	2	3_4	53 <u>4</u>	$6\frac{1}{2}$	61/2	$6\frac{3}{4}$	63/4	7	5 3/4
7/8	11/2	13/4	13/4	2	2		57/8	$6\frac{1}{2}$	63/4	634	7	7	$5\frac{7}{8}$
1	134	134	2	2	21/4	1	G	63/4	63	7	7	71/4	6
1 1/8	1 3/4	2	2	$2\frac{1}{4}$	$2\frac{1}{4}$	18	61/8	6 ³ / ₄	7	7	71/4	71/4	6 1/8
11/4	2	2	$2\frac{1}{4}$	$2\frac{1}{4}$	$2\frac{1}{2}$	111	$G_{\frac{1}{4}}$	7	7	71/4	71/4	71/2	61/4
13/8	2	$2\frac{1}{4}$	$2\frac{1}{4}$	21/2	$2\frac{1}{2}$	$1\frac{3}{8}$	63	7	71/4	71/4	71/2	$7\frac{1}{2}$	63
$1\frac{1}{2}$	21/4	$2\frac{1}{4}$	$2\frac{1}{2}$	$2\frac{1}{3}$	$2\frac{3}{4}$	11/2	$6\frac{1}{2}$ $6\frac{5}{8}$	$7\frac{1}{4}$	$7\frac{1}{4}$	71/2	71/2	73	$6\frac{3}{6}$ $6\frac{1}{2}$ $6\frac{5}{8}$
$1\frac{5}{8}$	21/4	$2\frac{1}{2}$	21/2	234	$2\frac{3}{4}$	15/8	65	71/4	71/2	71/2	73/4	73/4	68
134	$2\frac{1}{2}$	$2\frac{1}{2}$	23/4	$2\frac{3}{4}$	3	134	$G_{\frac{3}{4}}$	7 1/2	7 ½	734	73	8	634
178	$\cdot 2\frac{1}{2}$	2 3/4	2 3/4	3	3	$1\frac{7}{8}$	$G\frac{7}{8}$	$7\frac{1}{2}$	73	73	8	8	67/8
2	2 3/4	2 3/4	3	3	31/4	2	7	73	734	8	8	81/4	7
2 1/8	23/4	3	3	31/4	31/4	2 1/8	7 1/8	73	. 8	8	81	81/4	$7\frac{1}{8}$ $7\frac{1}{4}$
21	3	3	31/4	$3\frac{1}{4}$	31/2	$2\frac{1}{4}$	$7\frac{1}{4}$ $7\frac{3}{8}$	8	8	81/4	81/4	81/2	74
$2\frac{3}{8}$	3	$3\frac{1}{4}$	31/4	$3\frac{1}{2}$	31/2	23/8	78	8	81/4	81/4	81/2	81/2	73
$2\frac{1}{2}$	31/4	31/4	31/2	$3\frac{1}{2}$	33/4	21/2	71/2	81/4	81/4	81/2	81/2	83	$7\frac{1}{2}$ $7\frac{5}{8}$
28 234	31/4	31/2	$3\frac{1}{2}$	$3\frac{3}{4}$	3 3 4	25	$7\frac{1}{2}$ $7\frac{5}{8}$	81/4	81/2	81/2	83	834	78
24	$3\frac{1}{2}$	31	3 3/4	3 3/4	4	23	$7\frac{3}{4}$	81/2	81/2	834	8 3 4	8	73
27/8	$3\frac{1}{2}$	3 3/4	$3\frac{3}{4}$	4	4	278	$7\frac{7}{8}$	81/2	8 <u>3</u>	834	9	8	778
3	3 3 4	3 3/4	4	4	41/4	3	8	83/4	8 3 4	9	9	91/4	8
3 1/8	3 3 4	4	4	41/4	41/4	3 1/8	$8\frac{1}{8}$ $8\frac{1}{4}$ $8\frac{3}{8}$	8 3 4	9	9	91	91/4	8 1/8
31/4	4	4	$4\frac{1}{4}$	41/4	41/2	31/4	81/4	9	9	91/4	91/4	91/3	81
33	4	41/4	41/4	41/2	$4\frac{1}{2}$	$3\frac{3}{8}$	83	9	91/4	91/4	91/2	91/2	838
31/2	414	41/4	41/2	41/2	434	3 1/2 3 5/8 3 3/4	81/2	91/4	91/4	91/2	91/2	9 3/4	8 1 2 8 5 8 8 3 4
3 5 8	41/4	$4\frac{1}{2}$	$4\frac{1}{2}$	43/4	434	38	85	91/4	91/2	91/2	9 3/4	934	85
34	$4\frac{1}{2}$	$4\frac{1}{2}$	$4\frac{3}{4}$	434	5	34	83	91/2	91/2	934	93	10	83/4
37/8	41/2	4 3/4	43/4	5	5	3 7/8	87/8	91.	93/4	93	10	10	87/8
4	434	43/4	5	5	51/4	4	9	93	93	10	10	101	9
4 1/8	4 ³ / ₄	5 5	5	51	54	4 1/8	$9\frac{1}{8}$ $9\frac{1}{4}$	934	10	10	101	101	9 1/8 9 1/4
41/4	5		51/4	5 1	5½	41/4	94	10	10	10½ 10½	101	101	03
43/8	5 _{1/4}	5 ¹ / ₄ 5 ¹ / ₄	5½ 5½	5½ 5½	5½ 5¾	43/8	93/8	10 10 ¹ / ₄	10 ¹ / ₄ 10 ¹ / ₄	101/2	10½ 10½	$10\frac{1}{2}$ $10\frac{3}{4}$	98
$4\frac{1}{2}$ $4\frac{5}{8}$	51/4	5½	5½	5 3 4	5 ³ / ₄	$4\frac{1}{2}$ $4\frac{5}{8}$	$9\frac{1}{2}$ $9\frac{5}{8}$	104	10 ¹ / ₂	10½ 10½	10%	104	9\frac{3}{8} 9\frac{1}{2} 9\frac{5}{8}
434	51/2	5½	534	534	6	48	$9\frac{3}{4}$	10½ 10½	10 ²	103	104	11	03
47/8	5½	5 3 4	5 34	6	6	47/8	$9\frac{7}{8}$	10 ½	102	103	11	11	$9\frac{3}{4}$ $9\frac{7}{8}$
5	534	53	6	6	61/4	5	10	103	104	11	11	1114	10
5 1/8	53	6	6	61/4	61/4	5 1/8	10 1/8 .	104	11	11	1111	1114	10 1/8
51/4	6	6	61/4	61	61/2	51	10 1/4	11	11	1114	1111	11 1/2	10 1/4
538	6	61/4	61/4	61/2	61/2	$\frac{5\frac{1}{4}}{5\frac{3}{8}}$	10 3/8	11	1114	1114	1112	1112	103/8
						9	•						0

RIVETS AND BOLTS. Lengths for Framing Beams.

		6	<u> </u>						-	96		-							€		
BIZE OF BEAM	WEIGHT PER FOOT	WEB	GRIP			BOLTB		GRIP	WEIGHT PER FOOT	SIZE OF BEAM	SIZE OF BEAM	WEIGHT PER FOOT	GRIP	RIVETS	BOLTS		RIVETS	GRIP	WEB	WEIGHT PER FOOT	SIZE OF BEAM
7	100.0	3 4	1 3 18	258	$2\frac{1}{4}$	23/4	31/4	15	100.0			40.0	17/18	27/6	$2\frac{1}{2}$	2	21/2	1	9 16	40.0	
	95.0	11 18	1 18	66	66	$2\frac{1}{2}$	31	1 9 16	95.0		10	85.0	1 3 6	$2\frac{7}{6}$	"	46	28	15	1/2	35.0	10
24	90.0	8	110	$2\frac{1}{2}$	2 "	0.1	0.7	$1\frac{1}{2}$	90.0	24		80.0	15 3	23/4	24	4 3	"	7 8 3	7 16 5	80.0	
	85.0	16	1 15	2 ³ / ₈	"	21/4	2 7 8	$1\frac{7}{16}$ $1\frac{3}{8}$	85.0	. "		25.0 85.0	1 3 1 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	$\frac{2^{\frac{5}{6}}}{2^{\frac{7}{8}}}$	$2\frac{1}{2}$	$1\frac{3}{4}$	$2\frac{1}{4}$ $2\frac{1}{2}$	1	9 16	25.0 35.0	
	100.0	7 8	18	234	$2\frac{1}{4}$	23/4	3 8	1 3 4	100.0		0	30.0	1 1 1 4	23	21/4	13/4	21/4	13 16	16	30.0	
	95.0	25 32	1 3 18	25	"	"	31/4	1 5 8	95.0		9	25.0	"	"	"	"	"	"	3 8	25.0	9
	90.0	23 32	1 ½	"		$2\frac{1}{2}$	31/8	1 9 16	90.0			21.0	1 1 8	$2\frac{5}{8}$	66	"	$2\frac{1}{8}$	11 16	9 32	21.0	
20	85.0	11 16	46	"	"	66	"	44	85.0	20	1 (-)	25.5	150	234	24	2	28	7 6	7 18	25.5	
10	80.0 75.0	8	116	$2\frac{1}{2}$	2	"	"	1 1/2	80.0		8	23.0	13/16	25		13/4	2 ¹ / ₄	3 4	11	23.0	8
	70.0	9 16	1	66	66	46	$2\frac{7}{8}$	1 7 16	70.0			18.0	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	"	"	"	21/8	11 16	32 1 4	18.0	
	65.0	1 2	15 18	$2\frac{3}{6}$	66	66	"	1 3 8	65.0			20.0	15/16	23/4	$2\frac{1}{4}$	2	23/6	7/8	7 16	20.0	
	90.0								90.0		7	17.5	13/16	$2\frac{5}{6}$	66	13/4	$2\frac{1}{4}$	3 4	11 32	17.5	7
	85.0								85.0		_	15.0	1 1 6	"	66	46	$2\frac{1}{6}$	11 18	1/4	15.0	
	80.0	21	. 1	-1			0.1	. 1	80.0	Ξ,	0	17.25	13	"	"	134	$2\frac{1}{4}$	13 16	1 <u>5</u> 32 11	17.25	e
18	75.0	21 32 9 16	1 1 1	$2\frac{1}{2}$	2 "	21/2	$3\frac{1}{6}$ $2\frac{7}{8}$	$1\frac{1}{2}$ $1\frac{7}{16}$	75.0	18	6	14.75	1 ¹ / ₁₆	$2^{\frac{1}{2}}$	2 "	"	2 1/8	11 18	32	14.75	6
	65.0	16	""	"		"	48	116	65.0			14.75	1 3 1 6	25	$2\frac{1}{4}$	1 3 4	$2\frac{1}{4}$	16 7 8	32	14.75	
	60.0	17 32	15 18	23/8	"	"	66	1 3 8	60.0		5	12.25	110	$2^{\frac{1}{2}}$	2	"	21/6	11 18	11 32	12.25	5
	55.0	15 32	7 8	66	**	$2\frac{1}{4}$	$2\frac{3}{4}$	15/18	55.0			9.75	1 <u>5</u>	$2\frac{3}{6}$	46	46	2	9 18	7 32	9.75	
	80.0	32	14	$2\frac{3}{4}$	24	$2\frac{3}{4}$	34	111	80.0			10.5	1 1 8	$2\frac{5}{6}$	$2\frac{1}{4}$	13/4	$2\frac{1}{4}$	3 4	3	10.5	
	75.0	32	1 1 1	$2\frac{5}{8}$	"	21/2	31/8	1 1 1	75.0		4	9.5	1110	$2\frac{1}{2}$	2	"	21/8	11 16	16	9.5	4
	70.0 65.0	5 6 21 32	110	21/2	2 "	"	,"	1 1 2 "	70.0 65.0	2		7.5	1	238	"		2	5 8	3	8.5	
15	60.0	1 2	15	$2\frac{3}{8}$		"	$2\frac{7}{8}$	1 3 6	60.0	15		7.5	1,10	21/2	2	"	$2^{\frac{1}{6}}$	16 11 16	11 32	7.5	
	55.0	"	"	"	"	46	"	"	55.0		3	6.5	1	"	66	16	66	5 6	1 4	6.5	3
	50.0	"	16	66	44	"	"	66	50.0			5.5	7/8	$2\frac{3}{8}$	66	"	2	1/2	32	5.5	
	45.0		7 8 13 18	21	13/4	21/4	2 3 4 "	$1\frac{5}{16}$ $1\frac{1}{4}$	45.0 42.0			Rivet	s an	d bol	lts a	re 3′	diar	n.			
	65.0		18	$2\frac{1}{4}$ $2\frac{3}{4}$	21/4	$2\frac{3}{4}$	31/4	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		-	-	All di	men	sions	s in	inch	es.				
	60.0	1	1 1 1 8	25/8	"	$2\frac{1}{2}$	31/8	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1											, E
	55.0		1	$2\frac{1}{2}$	2	"	$2\frac{7}{8}$	17/18	55.0												4
12	50.0		"	"	"	"	46	66	50.0	1 12											
	45.0		7	" 03	66	"	"	4.5	45.0												
	40.0 35.0	"	8	2 ³ / ₆	66	24	23/4	1 1 10	40.0 35.0	1											
	31.5	1	3 4	$2\frac{1}{4}$	13/4	"	25	1 3 18		1											17

BOLTS.

Dimensions for Heads, Nuts, etc., etc.

	1														
	N E	ROU	סאנ				HEAD					NUT			
	111-				HEXA	BONAL	HEX. OR	8QU.	ARE	squ	ARE	HEX. OR	HEXA	GONAL	
ETER	9		111111			1			۷.		7	11,111	-	3	ETER
DIAMETER					()					2		(DIAMETER
1		o d	TO QV	DE LCH	0 -	t a	÷	t ;	0 -	m ·	ь.	Ļ.	E .		0
	AREA	DIAM. AT ROOT OF THREAD	AREA AT ROOT OF THREAD	NO. OF THREADS PER INCH	LONG DIAM.	SHORT DIAM.	НЕІВНТ	SHORT DIAM.	LONG DIAM.	LONG DIAM.	SHORT DIAM.	неіснт	SHORT DIAM.	LONG DIAM.	
INS.	8Q. INS.	INS.	8Q. INS.		IN8.	INS.	INS.	INS.	INS.	IN8.	INS.	INS.	INS.	INS. 19	INS.
1 4	.049	.185	.027	20	7 16	3 8		3 8	1/2	32	2	1 4	1/2	32	1/4
3 8	.110	.294	.068	16	32	9 16		17 32 23	3 4	133	11 16	3 8	11 16	16	38
2	.196	.400	.126	13	7 8	3 4		32	1	$1\frac{1}{4}$	7 8	1/2	7 8	1	1 2 5 8
5/8	.307	.507	.202	11	$1\frac{3}{32}$	15		32	1 9 32	1 1 2	$1\frac{1}{16}$	5 8	1 1/16	132	8
3	.442	.620	.302	10	1 5 16	1 1/8	8	$1\frac{3}{32}$	1 1 1 7 3 2	125	$1\frac{1}{4}$	3 4	$1\frac{1}{4}$	134	34
7/8	.601	.731	.420	9	$1\frac{17}{32}$	1 1 1 1 1 1 1 1 1	3 4	$1\frac{9}{32}$	113	$2\frac{1}{32}$	17/16	8	$1\frac{7}{16}$	1 32	78
1	.785	.837	.550	8	13/32	1 1/2	7 8	1 15 32	$2\frac{1}{16}$	$2\frac{5}{16}$	1 8	1	1 5 8	1-78	1
13	.994	.940	.694	7	$1\frac{31}{32}$	$1\frac{11}{16}$	1	1 5 8	$2\frac{5}{16}$	$2\frac{9}{16}$	1 13	1-1/8	$1\frac{13}{16}$	$2\frac{3}{32}$	118
114	1.227	1.065	.893	66	$2\frac{3}{16}$	1 7/8	1 1 8	113	2 18	$2\frac{27}{32}$	2	$1\frac{1}{4}$	2	25/16	14
$1\frac{3}{8}$	1.485	1.160	1.057	6	2 <u>5</u>	2	$1\frac{1}{4}$	2	$2\frac{13}{16}$	3 3 3 2	$2\frac{3}{16}$	13/8	$2\frac{3}{16}$	2 17 32	$1\frac{3}{8}$
11/2	1.767	1.284	1.295	"	$2\frac{5}{8}$	$2\frac{1}{4}$	1 3 8	$2\frac{3}{16}$	$3\frac{1}{18}$	$3\frac{3}{8}$	$2\frac{3}{8}$	$1\frac{1}{2}$	$2\frac{3}{8}$	23/4	12
15/8	2.074	1.389	1.515	5 1/2	$2\frac{13}{16}$	$2\frac{7}{18}$	$1\frac{1}{2}$	$2\frac{3}{8}$	311/32	3 8	2 18	1 5 8	$2\frac{9}{16}$	$2\frac{31}{32}$	15/8
$1\frac{3}{4}$	2.405	1.491	1.746	5	$3\frac{1}{32}$	$2\frac{5}{8}$	1 5 8	$2\frac{17}{32}$	$3\frac{19}{32}$	$3\frac{29}{32}$	$2\frac{3}{4}$	13/4	$2\frac{3}{4}$	3 3 16	$1\frac{3}{4}$
178	2.761	1.616	2.051	6.6	31/4	$2\frac{13}{16}$	13/4	$2\frac{23}{32}$	3 27 32	$4\frac{5}{32}$	$2\frac{15}{16}$	1 7 8	$2\frac{15}{16}$	3 13 2	$1\frac{7}{8}$
2	3.142	1.712	2.302	$4\frac{1}{2}$	3 15 32	3	17/8	$2\frac{29}{32}$	41/8	4 7/16	31/8	2	31/8	3 5/8	2
$2\frac{1}{4}$	3.976	1.962	3.023	66	$3\frac{29}{32}$	3 3 8	$2\frac{1}{8}$	$3\frac{1}{4}$	45/8	$4\frac{31}{32}$	$3\frac{1}{2}$	$2\frac{1}{4}$	$3\frac{1}{2}$	41/18	$2\frac{1}{4}$
$2\frac{1}{2}$	4.909	2.176	3.719	4	$4\frac{11}{32}$	3 3/4	$2\frac{3}{8}$	$3\frac{5}{8}$	5 1/8	$5\frac{1}{2}$	378	$2\frac{1}{2}$	3-7/8	$4\frac{1}{2}$	$2\frac{1}{2}$
23/4	5.940	2.426	4.620	66	$4\frac{25}{32}$	4 1/8	$2\frac{5}{8}$	4	$5\frac{21}{32}$	6	$4\frac{1}{4}$	$2\frac{3}{4}$	$4\frac{1}{4}$	$4\frac{29}{32}$	$2\frac{3}{4}$
3	7.069	2.629	5.428	31/2	5 7/32	$4\frac{1}{2}$	$2\frac{7}{8}$	$4\frac{11}{32}$	65/32	6 9 16	45/8	3	4 5/8	5 3/8	3
$3\frac{1}{4}$		2.879	6.510	"	5 5 8	$4\frac{7}{8}$	$3\frac{1}{8}$	$4\frac{23}{32}$	$6\frac{21}{32}$	$7\frac{1}{16}$	5	31/4	5	5 13 5 16	$3\frac{1}{4}$
$3\frac{1}{2}$	9.621	3.100	7.548	31/4	$6\frac{1}{16}$	5 1/4	$3\frac{3}{8}$	5 1/16	$7\frac{3}{16}$	7 5 8	5 3/8	31/2	$5\frac{3}{8}$	61/8	$3\frac{1}{2}$
334	11.045	3.317	8.641	3	$6\frac{1}{2}$	5 5 8	3 5 8	5 7/16	$7\frac{11}{16}$	81/8	$5\frac{3}{4}$	$3\frac{3}{4}$	$5\frac{3}{4}$	6 21 2	$3\frac{3}{4}$
4	12.566	3.567	9.963	66	615	6	3 7/8	5 13	8 7 2	8 21 32	61/8	4	61/8	$7\frac{3}{32}$	4
$4\frac{1}{4}$			11.329	$2\frac{7}{8}$	$7\frac{11}{32}$	63/8	$4\frac{1}{3}$	65/32	823	93/16	$6\frac{1}{2}$	$4\frac{1}{4}$	61	7 9 16	$4\frac{1}{4}$
$4\frac{1}{2}$			12.753	23/4	7^{25}_{32}	$6\frac{3}{4}$	$4\frac{3}{8}$	6 32	9 7 32	9 3	6 7	$4\frac{1}{2}$	678	$7\frac{31}{32}$	$4\frac{1}{2}$
$4\frac{3}{4}$			14.226	$2\frac{5}{8}$	8 7/32	$7\frac{1}{8}$	4 <u>5</u>	$6\frac{7}{8}$	9 3/4	10 1/4	$7\frac{1}{4}$	$4\frac{3}{4}$	$7\frac{1}{4}$	8 13 32	$\frac{13}{4}$
5	-		15.763	$2\frac{1}{2}$	821/32	$7\frac{1}{2}$	$4\frac{7}{8}$	$\frac{7\frac{1}{4}}{}$	10 1/4	$10\frac{25}{32}$	75/8	5	7 5/8	827/32	5
$5\frac{1}{4}$			17.572	46	932	$7\frac{7}{8}$	5 ½ 5 ½	75/8	$10\frac{3}{4}$	10_{32} $11\frac{3}{8}$	8	$5\frac{1}{4}$	8	$9\frac{9}{32}$	$5\frac{1}{4}$
$5\frac{1}{2}$		1	19.267	238	$9\frac{1}{2}$	81/4	5 3 8	$7\frac{31}{32}$	$10\frac{1}{4}$ $11\frac{9}{32}$	118	83/8	$5\frac{1}{2}$	838	$9\frac{23}{32}$	
$\begin{bmatrix} 3\frac{3}{4} \\ 5\frac{3}{4} \end{bmatrix}$			21.262	66	9 15 18	85/8	5 5 8	8 11 8 22	$11\frac{25}{32}$	1	834	$5\frac{3}{4}$	834	$10\frac{5}{32}$	5½ ×3
	1									12 ³ / ₈					$5\frac{3}{4}$
6	28.274	0.423	23.098	21/4	103/8	9	5 7/8	8 ²³ / ₃₂	12 ⁵ / ₁₈	$12\frac{15}{16}$	91/8	6	91/8	1032	6
							2:	2							

STAGGERING OF RIVETS.

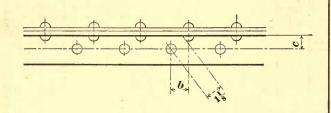
DISTANCE C. TO C. OF STAGGERED RIVETS.

VALUES	UE "X"	FOR VARY	ING VALUE	SOF "A" A	ND "R"

	VALUES	INCHES	INCHES	INCHES	INCHES	INCHES	INCHES	ALUES	OF A	INCHES	INCHES	INCHES	INCHES	INCHES	INCHES
	B INCHES	78	1	1 1 8	$1\frac{1}{4}$	18	11/2	15/8	1 4	178	2	$2\frac{1}{8}$	$2\frac{1}{4}$	$2\frac{8}{8}$	21/2
	11/8	1 7/16	11/2	1 16,	111	134	17	2	2 1/18	2 3 18	$2\frac{5}{18}$	28	21/2	2 5	2.3
	11/4	1 16	1 5 8	111/16	13/4	1 7 8	115	21/16	21/8	21/4	23	27 18	2 %	2 11/18	213
5	18	1 8	1118	134	178	1 15 18	2	21/8	$2\frac{3}{16}$	2 18	27	$2\frac{1}{2}$	2 5 8	2 3/4	2 7 8
	11/2	134	1 13 18	178	115	2	21/6	$2\frac{3}{18}$	$2\frac{6}{16}$	23	21/2	25/8	2 11/18	213	$2\frac{15}{16}$
P	$1\frac{5}{8}$	178	17/8	2	21/18	21/8	$2\frac{3}{16}$	25/16	28	21/2	2 16	211/16	23/4	2 7	3
	$1\frac{8}{4}$	1 15 18	2	210	218	23	2 5 16	23/8	2 10	2 18	2 8	234	2 7/8	215	310
	18	$2\frac{1}{18}$	21/8	$2\frac{3}{18}$	21/4	25/16	2 3	$2\frac{1}{2}$	2 18	$2\frac{5}{8}$	23/4	2 ¹³ / ₁₈	2 15 18	3	3 1/8
	2	$2\frac{3}{16}$	21/4	$2\frac{5}{16}$	238	2 18	21/2	2 9 18	2 5 8	234	213/16	215	3	3 1/8	3 3 16
	21/8	$2\frac{5}{16}$	$2\frac{5}{18}$	238	27	$2\frac{1}{2}$	25	211	23/4	$2^{\frac{13}{16}}$	2 15 18	3	3 18	3 3	3 1/4
	$2\frac{1}{4}$	27/18	2 7/18	$2^{\frac{1}{2}}$	2 16	25/8	211/18	23/4	2 7/8	$2\frac{15}{16}$	3	318	3 18	34	3 8
A	28/8	$2\frac{1}{2}$	2 18	258	211/16	2 3 4	2 13	278	215	3	318	3 3	31	3 8	37
	21	2 8	2 11 18	23/4	213	24	$2\frac{15}{16}$	3	31/16	31/8	3 %	31/4	3 8	37	3 18
Nome 1				14.0											

NOTE: Values below or to right of upper zigzag lines are large enough for % rivets.

Minimum Stagger for Rivets.



,	6 "	
c	3 DIAM.	7 DIAM.
INCHES	INCHES	D INCHES
1 1/8	1 1/4	138
1 16	1 3 16	15
$1\frac{1}{4}$	1 ½	1 1.
$1\frac{5}{16}$	1 10	1 3 16
18	1 <u>5</u> 16	1 1 8
$1\frac{7}{16}$	78	1
12	. 3	15 16
$1\frac{9}{16}$	<u>5</u>	1 <u>3</u>
1 5/8	3 8	11 18
1116	0	1/2
$1\frac{3}{4}$	0	<u>5</u> 16

RIVETSPACING IN ANGLES, CLEARANCE FOR DRIVING ETC.

All Dimensions in Inches.



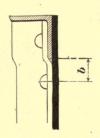


U							
LEG	G	MAX. RIVETS	LEG	G_1	G_2	MAX. RIVETS	
8	$4^{\frac{1}{2}}$	7 8	8	3	3	7 8	
7	4		7	$2\frac{1}{2}$	3	. "	
6	3 1/2	**	6	21/4	21/2	"	
5	3	. "	5	2	13/4	"	
4	21/4						
3 1/2	2	"	When $6'' \perp \text{ Exceeds } \frac{3''}{4}$				
3	1 3/4	"	6	21/2	21/4	7 8	
$2\frac{3}{4}$	158	3 4					
$2\frac{1}{2}$	13/8	5 8					
21/4	1 1/4	**					
2	11/8	1 2					
13/4	1						
14	7 8	3 8					
11/4	34						
1	9 16	1/4					
		7					

Minimum Rivet Spacing

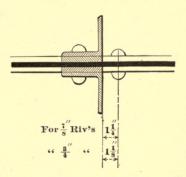
SIZE OF RIVET	1/4	3 8	1 2	5 8	34	7 8	1
MIMIMUM DISTANCE	1	11/4	13/4	2	$2\frac{1}{4}$	$2\frac{5}{8}$	3

Rivets in Crimped Angles



When angles are crimped, distance "b" should be $1\frac{1}{2}$ " plus twice thickness of chord angles, but never less than 2''

Clearance for Rivetting



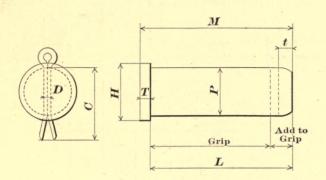
PINS WITH LOMAS NUTS.

All dimensions in inches.

Z Z	PIN					NUT			
DIAMETER OF PIN	sc	REW	ADD	STANDARD DIMENSIONS	DIAM. OF	SHORT	LONG	WEIGHT	DIAMETER OF PIN
ā	DIAM.	LENGTH	TO GRIP	6 Threads per inch.	ROUGH HOLE	DIAM.	DIAM.	IN LBS.	DIA
0	$1\frac{1}{2}$	1-1/2	1/4		1 6 10	31/4	334	2.5	2
2			1 4	1½".	1 16	$3\frac{1}{4}$	3 3 4	2.5	
$2\frac{1}{4}$	$1\frac{1}{2}$	1 1 1	1 4		1 13 16	3 3 4			$2\frac{1}{4}$
$2\frac{1}{2}$	2	11/2					4 5	2.5	$\frac{2^{1}}{2}$
$2\frac{3}{4}$	2	11/2	1 1		1 13 16	3 3 4	4 5 18	2.5 ^	$2\frac{3}{4}$
3	$2\frac{1}{2}$	11/2	1 4	$G \longrightarrow \emptyset$	$2\frac{5}{16}$	$4\frac{1}{2}$	5 3 18	3.0	3
$3\frac{1}{4}$	$2\frac{1}{2}$	1 1 2	1 1	$\begin{vmatrix} 1 \\ \frac{3}{8} \end{vmatrix}$	$2\frac{5}{18}$	$4\frac{1}{2}$	5 3 16	3.0	31
$3\frac{1}{2}$	$2\frac{1}{2}$	1 1 2	1/4	•	$2\frac{5}{16}$	41/2	5 3 18	3.0	$3\frac{1}{2}$
$3\frac{3}{4}$	3	17/8	1/2		213/16	5	5 3/4	5.5	$3\frac{3}{4}$
4	3	1 7 8	1 2	17"	213/16	5	5 3/4	5.5	4
$4\frac{1}{4}$	31/2	17/8	1/2	1 ⁷ / ₃	3 16	5 3/4	6 8	7.0	$4\frac{1}{4}$
$4\frac{1}{2}$	31/2	1 7 8	1 2		3 18	5 3/4	6 5 B	7.0	$4\frac{1}{2}$
$4\frac{3}{4}$	31/2	1 7 8	1/2		3 18	5 3 4	6 5 e	7.0	$4\frac{3}{4}$
5	4	178	1/2		318	61/2	$7\frac{1}{2}$	8.5	5
$5\frac{1}{4}$	4	1 7 8	1/2	G	318	61/2	$7\frac{1}{2}$	8.5	$5\frac{1}{4}$
$5\frac{1}{2}$	$4\frac{1}{2}$	1 7 8	1/2	11 S S S S S S S S S	$4\frac{5}{16}$	7	8 1/8	11.0	$5\frac{1}{2}$
$5\frac{3}{4}$	$4\frac{1}{2}$	1 7 8	1/2	$\frac{1}{2}$	4 5 18	§ 7	81/8	11.0	$5\frac{3}{4}$
6	$4\frac{1}{2}$	1 7 8	1/2		4 5 16	7	8 1/8	11.0	6
$6\frac{1}{4}$	5	23/8	3 4		4 13	73/4	8 15 16	12.0	$6\frac{1}{4}$
$6^{\frac{1}{2}}$	5	23	3 4		413 16	73/4	815	12.0	$6\frac{1}{2}$
$6\frac{3}{4}$	5 ½	2 ³ / ₆	3 4		5 5 16	81/4	91/2	13.5	$6\frac{3}{4}$
7	5 1/2	238	3 4	71	5 5 10	81/4	91/2	13.5	7
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	5 1/2	$2\frac{3}{8}$	3 4	2 ^{3''} ⊭←->⊭	5 5 18	81/4	9 1/2	13.5	71
7 1 2	5 1/2	2 <u>3</u>	3 4	-¢-°≯	5 18	81/4	91/2	13.5	7 1/2
734	6	$2\frac{3}{6}$	3 4		5 13 18	9	10 3/8	17.0	73
8	6	23/8	3 4		5 13 16	9	10 3	17.0	8
81/4	6	$2\frac{3}{8}$	3 4		5 18	9	10 3 e	17.0	81/4
$8^{\frac{1}{2}}$	6	23/8	3 4		5 13 18	91/2	10 7 8		$8\frac{1}{2}$
$8\frac{3}{4}$	6	23/8	3 4	$G \longrightarrow S$	5 13 16	101	12 ½		$8\frac{3}{4}$
9	6	23/8	3 4	12	$5\frac{13}{16}$	101/2	12 ¹ / ₈		9
$9^{\frac{1}{4}}$	6	$2\frac{\dot{s}}{s}$	3 4	37	5 18 18	101/2	12 ¹ / ₆		$9^{1\over4}$
$\begin{array}{c c} 9^{\frac{1}{4}} \\ 9^{\frac{1}{2}} \end{array}$									$9\frac{1}{2}$
$9^{\frac{3}{4}}$									$rac{9rac{1}{2}}{9rac{3}{4}}$
10	PE .								10
NOTE:- To obtain grip "G" add 1 for each bar, together with amount given in table,									

NOTE:- To obtain grip "G" add $\frac{1}{16}$ for each bar, together with amount given in table,

COTTER PINS.



All dimensions in inches.

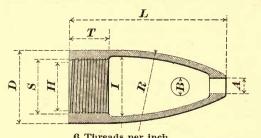
		PIN		НЕ	AD	СОТ	TER	ADD T	O GRIP	
OF PIN	DIAMETER OF PIN-HOLE	TAPE	ER AT END $oldsymbol{t}$	DIAMETER	THICKNESS	LENGTH C	DIAMETER D	FOR LENGTH OVER ALL M	FOR LENGTH UNDER HEAD	DIAMETER OF PIN
1				n n	-					1
1	$1\frac{1}{32}$	16	X 1/16	11/4	1/4	1 3/4	1/4	78	5 8	1
$1^{\frac{1}{4}}$	1 9/32	5 16	X 1/16	11/2	1/4	2	1/4	7 8	5 8	$1\frac{1}{4}$
$1\frac{1}{2}$	1 17/32	7 16	X 3	13/4	1/4	$2\frac{1}{2}$	5 13	1 1 8	7 8	$1\frac{1}{2}$
$1\frac{3}{4}$	1 25 32	7 16	X 32	2	1/4	$2\frac{3}{4}$	5 16	1 1 8	7 8	$1\frac{3}{4}$
2	$2\frac{1}{32}$	1 2	x 1/8	$2\frac{3}{8}$	3 8	3	3 8	1 3 8	1	2
$2\frac{1}{4}$	$2\frac{9}{32}$	1 2	x 1/8	25/8	3 8	$3\frac{1}{4}$	3 8	13/8	1	$2\frac{1}{4}$
$2\frac{1}{2}$	$2\frac{17}{32}$	8	X	$2\frac{7}{8}$	3 ** 8	3 3/4	7 18	1 1 2	1 1 8	$2\frac{1}{2}$
$2\frac{3}{4}$	$2\frac{25}{32}$	5 8	x	31/8	3 8	4	7 16	11/2	1 1 8	$2\frac{3}{4}$
3	3 1 32	3 4	x 3/16	31/2	1/2	5	1/2	1 7 8	13/8	3
31/4	3 8 32	3 4	X 3/16	3 3 4	1 2	5	1/2	174	1 3 8	$3\frac{1}{4}$
$3\frac{1}{2}$	3 17 32	7 8	x	4	1/2	6	5 8	21/8	1 5 8	$3\frac{1}{2}$
$3\frac{3}{4}$	3 25 32	7 8	X,	41/4	1/2	6	5 8	21/8	1 5 8	$3\frac{3}{4}$
										ō

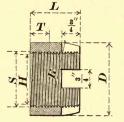
NOTE:- Use pins with lomas nuts, in preference to cotter pins, whenever possible.

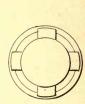
PILOT NUTS.

Cast Steel.

All dimensions in inches.







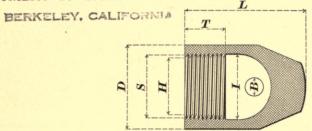
6 T	iread	s per	inch.
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$ \begin{array}{c c c c c c c c c c c c c c c c c c c $				(3 Tire:	ads per	inch.				6	Thread	ls per	inch.			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	DIAM.	SCRE							DIAM. HOL	PAT. NO.	PAT. NO.		L STRAIGHT PORTION	T COVER ALL		DIAM. OF SCREW	DIAM.
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	2	11/2	1 5 16	1 5 8	5	1 8	8 1/2	1 2	1 2	L 8	S 8	$4\frac{3}{4}$	3 4	2	1 8 16	11/2	2
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	21/4	11/2	1 5 18	1 5/8	51/4	1 8	8 1/2	1/2	1 2	L 9	S 9	31/2	3 4	2	1 8 16	1 1/2	$2\frac{1}{4}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$2\frac{1}{2}$	2	1 13	1 5/8	5 3/4	21/8	101	8	8	L10	S10	$4\frac{3}{4}$	3 4	2	1 13	2	$2\frac{1}{2}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$2\frac{3}{4}$	2	1 13	1 8	6	$2\frac{1}{4}$	$10^{\frac{1}{2}}$	5 8	8	L11	S11	31/2	3 4	2	1 13	2	$2\frac{3}{4}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	3	$2\frac{1}{2}$	2 5 18	1 5/8	$6\frac{1}{2}$	$2\frac{\delta}{8}$	12	3 4	3 4	L12	S12	$4\frac{3}{4}$	3 4	2	$2\frac{5}{16}$	$2\frac{1}{2}$	3
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		$2\frac{1}{2}$	$2\frac{5}{16}$	1 8	6 3/4	$2\frac{3}{4}$	12	3 4	3 4	L13	S13	31/2	3 4	2	$2\frac{5}{16}$	$2\frac{1}{2}$	$3\frac{1}{4}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		$2\frac{1}{2}$		1 5 e	7	3	12			L14	S14	$2\frac{1}{2}$	3 4	2		$2\frac{1}{2}$	$3\frac{1}{2}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$3\frac{3}{4}$	3	$2\frac{13}{16}$	2	81/4	31/8	$15\frac{1}{2}$	7 8		L15	S15	$4\frac{1}{2}$	1	$2\frac{1}{2}$	$2\frac{13}{16}$	3	$3\frac{3}{4}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4	3	2 13 16	2	81/2	$3\frac{3}{8}$	151	7 8	7 6	L16	S16	$3\frac{1}{2}$	1	$2\frac{1}{2}$	2 13 16	3	4
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	41	31/2	3 10	2	8 3/4	3 8	$15\frac{1}{2}$	7 8	7/8	L17	S17	$4\frac{1}{2}$	1	$2\frac{1}{2}$	3 16	31/2	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		$3\frac{1}{2}$	3 8 16	2	9	3 3/4	$15\frac{1}{2}$			L18	S18	$3\frac{1}{2}$	1	$2\frac{1}{2}$	3 16	31/2	$4\frac{1}{2}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$4\frac{3}{4}$	3 1/2		2	9 1/4	4	$15\frac{1}{2}$	7 8	7 8	L19	S19	3	1	$2\frac{1}{2}$	3 16	$3\frac{1}{2}$	$4\frac{3}{4}$
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		4	100	2	91/2	$4\frac{1}{4}$	161/2	1	1	L20	S20	4	1	$2\frac{1}{2}$	3 18	4	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		4	3 18	2	9 3/4	$4\frac{1}{2}$	$16\frac{1}{2}$	1	1	L21	S21	3	1	$2\frac{1}{2}$	3 18	4	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$5\frac{1}{2}$	$4\frac{1}{2}$	4 5 18	2	101/2	4 5 e	$20\frac{1}{2}$	11/4	1	L22	S22	4	1	$2\frac{1}{2}$	$4\frac{5}{16}$	41/2	$5\frac{1}{2}$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$5\frac{3}{4}$	41/2		2	10 ³ / ₄	4 7 8	$20\frac{1}{2}$	11/4	1	L23	S23	3	1	$2\frac{1}{2}$	4 16		$5\frac{3}{4}$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		$4^{\frac{1}{2}}$		2		5 1/8	$20\frac{1}{2}$	11/4	1	L24	S24	$2\frac{1}{2}$	1	$2\frac{1}{2}$	4 16	$4^{\frac{1}{2}}$	
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		5		$2\frac{1}{2}$	$11\frac{3}{4}$	5 3 6	$20\frac{1}{2}$	11/4	1	L25	S25	$4\frac{1}{2}$	11/4	3	4 13 16	5	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			1				1		1	L26	S26	$3\frac{1}{2}$		3			$6\frac{1}{2}$
						5 3/4	$20\frac{1}{2}$		1	L27	S27			3			$6\frac{3}{4}$
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$							21		1	L28	S28	31/2		3			
$7\frac{3}{4}$ 6 $5\frac{18}{16}$ $2\frac{1}{2}$ $13\frac{1}{2}$ 6 6 2 23 $.1\frac{1}{2}$ $1\frac{1}{4}$ L31 S31 $3\frac{1}{2}$ $1\frac{1}{4}$ 3 $5\frac{18}{16}$ 6 $7\frac{3}{4}$		100			$12\frac{3}{4}$		21	11/4	1	L29	S29	3	14	3	5 18		71
							21			L30	S30			3		$5\frac{1}{2}$	7 1 2
							-	-			S31						
8 6 $5\frac{18}{16}$ $2\frac{1}{2}$ $13\frac{3}{4}$ $6\frac{3}{4}$ 23 $1\frac{1}{2}$ $1\frac{1}{4}$ L32 S32 $3\frac{1}{4}$ $1\frac{1}{4}$ 3 $5\frac{13}{16}$ 6 8			ł	12.00													
$egin{array}{ c c c c c c c c c c c c c c c c c c c$				i													
$egin{array}{ c c c c c c c c c c c c c c c c c c c$	0 2	g	5 16	$2\frac{1}{2}$	144	1/4	23	11/2	1 1/4	L34	\$34	$2\frac{3}{4}$	$1\frac{1}{4}$	3	5 18	6	8=
			247								- 4			- 4			
27														1/2 22			

DRIVING NUTS. Cast Steel.

UNIVERSITY OF CALIFORNIA

PARTMENT OF CIVIL ENGINEER



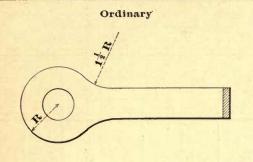
6 threads per inch

				2			
DIAMETER	DIAMETER OF SCREW	SIZE OF ROUGH HOLE	LENGTH OF THREAD	LENGTH OVER ALL	INSIDE DIAMETER	DIAMETER OF HOLE	PATNO.
D	S	H	T	\boldsymbol{L}	I	В	
INCHES	INCHES	INCHES	INCHES	INCHES	INCHES	INCHES	
$2\frac{1}{4}$	1 1 2	1-5-	1 5	4	1 ⁵ / ₈	1/2	D 3
$2\frac{3}{4}$	2	1 13 16	1 5 8	4 ³ / ₈	2 ¹ / ₈	5 8	D 4
$3\frac{1}{2}$	$2\frac{1}{2}$	$2\frac{5}{16}$	1 8	5	23/4	3 4	D 5
4	3	$2\frac{13}{16}$	2	5 1/4	31/4	7 8	D 6
$4\frac{3}{4}$	31/2	35/16	2	578	33/4	"	D 7
$5\frac{1}{4}$	4	313	2	61/4	$4\frac{1}{4}$	1	D 8
6	$4\frac{1}{2}$	45/16	2	63/4	$4\frac{3}{4}$	"	D 9
$6\frac{1}{2}$	5	413/16	$2\frac{1}{2}$	71/8	51/4	44	D10
7 1/2	5 ¹ / ₂	5 5 16	$2\frac{1}{2}$	7 8	534	44	D1,1
81/2	6	513/16	$2\frac{1}{2}$	778	61/4	46	D12

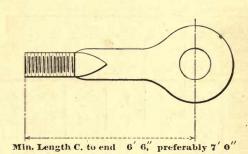
MAXIMUM BENDING MOMENTS ON PINS

	PIN	МОМЕ	NTS IN INCH POL	JNDS FOR FIBRE	STRAINS PER SQ.	IN. OF	PIN	
DIAM.	AREA	15,000	18,000	20,000	22,000	25,000	AREA	DIAM.
1	0.785	1470	1770	1960	2160	2450	0.785	1
11/4	1.227	2880	3450	3830	4220	4790	1.227	11/4
$1\frac{1}{2}$	1.767	4970	5960	6630	7290	8280	1.767	11/2
$1\frac{3}{4}$	2.405	7890	9470	10500	11570	13200	2.405	13
2	3.142	11800	14100	15700	17280	19600	3.142	2
$2\frac{1}{4}$	3.976	16800	20100	22400	24600	28000	3.976	21/4
$2\frac{1}{2}$	4.909	23000	27600	30700	33700	38400	4.909	21/2
$2\frac{3}{4}$	5.940	30600	36800	40800	44900	51000	5.940	23/4
3	7.069	39800	47700	53000	58300	66300	7.069	3
31/4	8.296	50600	60700	67400	74100	84300	8.296	31
$3\frac{1}{2}$	9.621	63100	75800	84200	92600	105200	9.621	31/2
$3\frac{3}{4}$	11.045	77700	93200	103500	113900	129400	11.045	334
4	12.566	94200	113100	125700	138200	157100	12.566	4
$4\frac{1}{4}$	14.186	113000	135700	150700	165800	188400	1/110	11/4
$4\frac{1}{2}$	15.904	134200	161000	178900	196800	223700	15.904	141
$4\frac{3}{4}$	17.721	157800	189400	210400	231500	263000	17.721	43
5	19.635	184100	220900	245400	270000	306800	19.635	5
$5\frac{1}{4}$	21.648	213100	255700	284100	312500	355200	21.648	51
$5\frac{1}{2}$	23.758	245000	294000	326700	359300	408300	23.758	$5\frac{1}{2}$
$5\frac{3}{4}$	25.967	280000	335900	373300	410600	466600	25.967	534
6	28.274	318100	381700	424100	466500	530200	28.274	6
61	30.680	359500	431400	479400	527300	599200	30.680	61
$6\frac{1}{2}$	33.183	404400	485300	539200	593100	674000	33.183	61/2
$6\frac{3}{4}$	35.785	452900	543500	603900	664200	754800	35.785	634
7	38.485	505100	606100	673500	740800	841900	38.485	7
71	41.282	561200	673400	748200	823000	935300	41.282	74
71/2	44.179	621300	745500	828400	911200	1035400	44.179	72
734	47.173	685500	822600	914000	1005300	1142500	47.173	73/4
8	50.265	754000	904800	1005300	1105800	1256600	50.265	8
81/4	53.456	826900	992300	1102500	1212800	1378200	53.456	81
81.	56.745	904400	1085200	1205800	1326400	1507300	56.745	81/2
83/4	60.132	986500	1183800	1315400	1446900	1644200	60.132	834
9	63.617	1073500	1288200	1431400	1574500	1789200	63.617	9
91	67.201	1165500	1398600	1554000	1709400	1942500	67.201	91/4
$9\frac{1}{2}$	70.882	1262600	1515100	1683400	1851800	2104300	70.882	91/2
93/4	74.662	1364900 ,	1637900	1819900	2001900	2274900	74.662	93
10	78.540	1472600	1767100	1963500	2159900	2454400	78.540	101
101	82.520	1585900	1903000	2114500	2325900	2643100	82.520	101
$10\frac{1}{2}$	86.590 90.760 *	1704700	2045700	2273000	2500200	2841200	86.590 90.760	102
$10\frac{3}{4}$		1829400	2195300	2439300	2683200	3049100		103
11	95.030	1960100	2352100	2613400	2874800	3266800	95.030	11
11_{4}^{1}	99.400	2096800	2516100	2795700	3075400	3494800	99.400	111
111/2	103.870	2239700	2687600	2986300	3284800	3732800	103.870	1112
113	108.430	2388900	2866600	3185200	3503700	3981500	108.430	113
12	113.100	2544700	3053600	3392900	3732190	4241200	113.100	12

EYE BARS







WIDTH	MIN. THICKNESS		HEAD		sc	REW END		THICKNESS	WIDTH
BAR	OF BAR	DIAM.	MAX. PIN.	ADD'L MATERIAL FOR HEAD	ADD'L MATERIAL FOR UPSET	DIAM.	LENGTH	OF BAR	BAR
40000	BL = INS.	INS.	1N6.	FT. & INS.	FT. & INS.	IN6	INS.	INS.	INS.
2	<u>8</u>	$4\frac{1}{2}$	1 3	$0 - 7\frac{1}{2}$	0-7	2	5	15	2
~	**	$5\frac{1}{2}$	$2^{\frac{3}{4}}$	1 - 0 1/2	0-7	4	3	15 16	14
2 1/2	3 4	5½	$2\frac{1}{4}$	0 - 91/2	1-1	$2\frac{1}{4}$	5	15 to 1 1 1	$2\frac{1}{2}$
~ 2	44	$6\frac{1}{2}$	3 1/4	$1 - 1\frac{1}{2}$					~ 2
3	3 4	7	3	1-3	1-5	$2^{\frac{1}{2}}$	5 1/2	1 to 116	3
9	**	8	4	1-6	1- 5	2 3/4	6	11 to 11	
4	3 4	91/2	$4^{\frac{1}{4}}$	1-8	1-8	3	6	1 to 11/8	4
-	- "	101/2	$5\frac{1}{4}$	1 - 10	1-8	3 1/4	61/2	$1\frac{3}{16}$ to $1\frac{3}{8}$	æ
5	3 4	1112	5	1- 9	1- 9	3 1/4	$6\frac{1}{2}$	1 to 116	5
0	1	$12^{\frac{1}{2}}$	6	2-1	1 - 9	3 ½	7	11 to 11	
6	3 4	13½	5 1/2	1-11	1-11	3 3/4	8	11 to 13	6
	1	$14^{\frac{1}{2}}$	61/2	2 - 2	1-11	4	8	11 to 13	U
7	7 8	16	6 3/4	2-3	2-3	4 4	9	11 to 15	7
	18 18	17	73/4	2-8	2-3	4 ½	9	13 to 11	
	1	17	$6^{\frac{1}{2}}$	2-3					
8	1 1/16	18	$7\frac{1}{2}$	2-6					8
	1 1 8	181/2	8	2-10					
	1 1 8	19 ½	7 3/4	2-6					
9	44	$21\frac{1}{2}$	• 9 4	3 - 1					9
	1 3	22	9	2 - 11 '					
10	"	23	10	3 - 3	A 14 - 15 - 15 - 15 - 15 - 15 - 15 - 15 -				10
40									.40
12									12

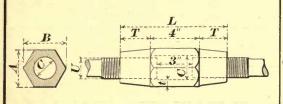
Note: Eye bars are hydraulic forged, and are guaranteed to develop the full strength of the bar, under conditions given in the above table, when tested to destruction.

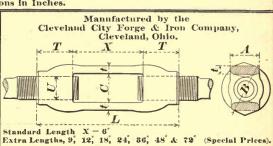
STANDARD UPSETS. For Round and Square Bars.

	F	ROUND	0	BARS				SQI	JARE		BARS		
RO	JND			UPSET					UPSET			squ	ARE
DIAM.	AREA	DIAM.	LENGTH	ADD	AREA AT ROOT	EXCESS AREA	EXCESS AREA	AREA AT ROOT	ADD	LENGTH	DIAM.	AREA	DIAM.
INCHES	SQ.INS.	INCHES	INCHES	INCHES	8Q. INS.	%	%	SQ. INS.	INCHES	INCHES	INCHES	8Q.1NS.	INCHES
5 8	0.307	7 8	4	$4\frac{1}{2}$	0.420	36.8	100						8
34	0.442	1	4	3 7/8	0.550	24.4	206	0.694	3 1/2	4	1 1/8	0.563	3 4
7/8	0.601	$1\frac{1}{4}$	4	5	0.891	48.3	16.3	0.891	4	4	1 1/4	0.766	7/8
1	0.785	13/8	4	$4\frac{3}{8}$	1.057	34.7	29.5	1.295	4	4	1 1/2	1.000	1
11/8	0.994	11/2	4	3 7/8	1.295	30.3	19.7	1.515	4 1/2	4 1/2	1 5/8	1.266	1 1 2 8
11/4	1.227	1 8	41/2	3 7/8	1.515	23.5	31.1	2.049	4 1/2	4 1/2	1 7 8	1.563	11/4
$1\frac{3}{8}$	1.485	13/4	$4\frac{1}{2}$	3 1/2	1.744	17.4	21.7	2.302	4 1/8	5	2	1.891	$1\frac{3}{8}$
11/2	1.767	2	5	4 5 8	2.302	30.3	34.0	3.023	4 3/4	5	2 1/4	2.250	11/2
15/8	2.074	21/8	5	41/4	2.651	27.8	29.6	3.410	4 5 8	5 ½	$2\frac{3}{8}$	2.641	15/8
$1\frac{3}{4}$	2.405	21/4	5	4	3.023	25.7	21.3	3.716	4 1/4	5 1/2	2 1/2	3.063	$1\frac{3}{4}$
$1\frac{7}{8}$	2.761	2 ³ / ₈	5 ¹ / ₂	41/8	3.410	23.9	31.4	4.619	5 1/8	6	$2\frac{3}{4}$	3.516	$1\frac{7}{8}$
2	3.142	$2\frac{1}{2}$	5 1/2	3 7/8	3.716	18.3	27.7	5.107	4 3/4	6	$2\frac{7}{8}$	4.000	2
21/8	3.547	$2\frac{5}{8}$	5 1/2	3 5 8	4.155	17.1	20.2	5.430	4 3/8	6	3	4.516	2 1/8
21/4	3.976	$2\frac{7}{8}$	6	$4\frac{5}{8}$	5.107	28.5	28.6	6.510	5 1/8	61/2	31/4	5.063	21/4
$2\frac{3}{8}$	4.430	3	6	4 3 8	5,430	22,6	33.8	7.548	6 ½	7	3 1/2	5.641	$2\frac{3}{8}$
$2\frac{1}{2}$	4.909	31/8	61/2	$4\frac{3}{8}$	5.957	21.3	30.7	8.170	6 1/4	8	3 5/8	6.250	21/2
25/8	5.412	31/4	61/2	4 1/4	6.510	20.3	35.0	9.305	6 ³ / ₄	8	3 7/8	6.891	$2\frac{5}{8}$
$2\frac{3}{4}$	5.940	3 3 8	7	41/4	7.088	19.3	32.1	9.994	6	8	4	7.563	$2\frac{3}{4}$
$2\frac{7}{8}$	6.492	3 8	8	5 1/2	8.170	25.9	37.0	11.329	8	9	41/4	8.266	$2\frac{7}{8}$
3	7.069	3 3 4	8 -	5 1/4	8.641	22.2	41.7	12.753	7 1/2	9	41/2	9.000	3
31/8	7.670	37/8	8	5 ½	9,305	21.3	Hel						$3\frac{1}{8}$
31/4	8.296	4	8	4 7 8	9.994	20.7					VX II		31/4
3 1/2	9.621	41/4	9	5 1/4	11.329	17.7							31/2
$3\frac{3}{4}$	11.045	41/2	9	4 3 4	12.753	15.5							334

SLEEVE NUTS AND TURNBUCKLES.

All Dimensions in Inches.





DIAM. OF 8CREW	LENGTH OF	LENGTH OF NUT	SHORT DIAM.	LONG DIAM.	INSIDE DIAM,	THICK- NESS	WEIGHT	WEIGHT	N LBS. t A B C L T				DIAM. OF SCREW		
U SCREW	THREAD	L	А	B	C	t	IN LBS.	IN LBS.	t	A	В	C	L	T	U SCREW
7 8	$1\frac{1}{2}$	7	15/8	178	118	$\frac{1}{4}$	$2\frac{3}{4}$	$2\frac{1}{2}$	3 8	$1\frac{1}{4}$	$2\frac{1}{4}$	1 1/4	85	1 5 18	78
1	11/2	7	1 5 a	17/8	1 ½	1/4	3	3 1/2	710		2 7 18	1 5 18	9	1 1/2	1
13	13/4	$7\frac{1}{2}$	2	2 6 10	1 ª	5 10	$3\frac{1}{2}$	4	1/2	**	2 9 16	1 7 18	98	1110	11/8
11/4	"	46	"			- "	4	51/4		112	2 3/4	1 10	93/4	1 7 8	11/4
$1\frac{3}{8}$	2	8	238	2 4	18	3 8	41/2	6	"	15/8	3 1 1 0	1 11 18	1018	$2\frac{1}{16}$	13/8
11/2	- 46	66	66		66		61/2	7	<u>5</u>	66	3 3 10	1-3	10½	$2\frac{1}{4}$	11/2
$1\frac{5}{8}$	21/4	81/2	234	3 3 16	1 7 8	7 18	8	8½		13/4	31/2	2	1078	$2\frac{7}{10}$	15/8
$1\frac{3}{4}$	66	46	46	"	**		81/2	10	"	2	3 3/4	21/8	$11\frac{1}{4}$	2 5 8	13/4
$1\frac{7}{8}$	$2\frac{1}{2}$	9	31/8	3 8	$2\frac{1}{8}$	1/2	10	11½	11 10	**	3 7/8	$2\frac{a}{16}$	11 8	213	17/8
2	44	66	66	66	14		11	13	**	$2\frac{1}{4}$	$4\frac{1}{4}$	2 8	12	3	2
21/8	23/4	$9^{\frac{1}{2}}$	$3\frac{1}{2}$	$4\frac{1}{10}$	2 ³ / ₈	9 10	14	15	23 32	$2\frac{1}{2}$	$4^{\frac{1}{2}}$	$2^{\frac{1}{2}}$	12ª	3 3 3	$2\frac{1}{8}$
$2\frac{1}{4}$	- 66	46	а	63	"	ec .	15	18	13	66	434	$2\frac{11}{16}$	12 ³ / ₄	3 8	21/4
$2\frac{3}{8}$	3	10	378	$4\frac{1}{2}$	258	5 8	18	20		2 3/4	$4\frac{7}{8}$	23/4	13½	3 2 16	23
$2\frac{1}{2}$	"	46	66	66	"		19	24	27 32	3	5 %	310	13½	3 4	$2\frac{1}{2}$
$2\frac{5}{8}$	34	101/2	$4^{\frac{1}{4}}$	415	$2\frac{7}{8}$	11 16	22	28	15	•6	5 %	3 1/8	13 ⁷ / ₈	315	25/8
$2\frac{3}{4}$	**	44	"		"	"	23	30	"	31/4	5 3/4	31/4	$14\frac{1}{4}$	4 1/8	$2\frac{3}{4}$
$2\frac{7}{8}$	$3\frac{1}{2}$	11	4 8	$5\frac{3}{8}$	31/8	3 4	27	34	1112	"	6 ½	3 7 10	148	4 5 16	$2\frac{7}{8}$
3	a			46	"	44	28	38	16.6	31/2	6 3	3 8	15	$4^{\frac{1}{2}}$	3
$3\frac{1}{8}$	33.	$11\frac{1}{2}$	5	513	38	13	34								$3\frac{i}{8}$
$3\frac{1}{4}$	ii,	66	"	"	66	G	35	50	110	4	6 3/4	378	15 ³	4 7 8	$3\frac{1}{4}$
$3\frac{3}{8}$	4	12	5 ³ / ₈	6 ½	35/8	7 8	39								$3\frac{3}{8}$
$3\frac{1}{2}$		60	- 44	"	**	"	40	65	$1\frac{7}{32}$	4	$7\frac{1}{4}$	$4^{\frac{1}{4}}$	$16\frac{1}{2}$	5 1/4	$3\frac{1}{2}$
$3\frac{5}{8}$	41/4	$12\frac{1}{2}$	5 34	611	37/8	15	45						97.11		$3\frac{5}{8}$
$3\frac{3}{4}$	"	34	46	46	66	66	47	11	1 5 10	5	81/4	$4\frac{7}{18}$	18	6	$3\frac{3}{4}$
$3\frac{7}{8}$	$4\frac{1}{2}$	13	6 ¹ / ₈	71/8	41/8	1	52	. 1	= ,				-		$3\frac{7}{8}$
4	66	46	46	**	**	66	55		17/10	5	8 3 4	45	18	6	4
41/4	434	13½	61/2	7 0	$4\frac{3}{8}$	110	65				-				41/4
$4\frac{1}{2}$	5	14	67/8	8	43/4	110	75		4.	Ä.					$4\frac{1}{2}$

LOOP RODS.

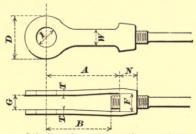
Allowance for Eye, Square or Round Bars.

All Dimensions in Inches.

	DIAM.	P NS	7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	टर टर टर टर मामानाव्यक्षाम	60 60 60 60 MINISTER	4 4 4 4	57 57 57 57 4 100 101 101 101 101 101 101 101 101 10	9	
		ಣ			27 ¹ 28 ¹ 29 30	3 22 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	34 35 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	38 8	
		200		18	2011 20 20 20 20 20 20 20 20 20 20 20 20 20	322 1 4 4 1 1 4 4 1 1 4 4 1 1 4 4 1 1 4 1 4 1 1 4 1 1 4 1 1 4 1	34 1 35 36 37	377	
		65		25 818	261 271 288	30 324 324	8 44 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	378	
		200		248	25 cc 26 cc 27 cc 28 cc	20 8 30 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	3341 35 35 36	37	
(P+R)		C5 H 05		23 1 24 8 24 8	25 8 2 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	29 30 31 31 ⁷	00 00 00 00 00 00 00 00 00 00 00 00 00	36 21	et.
Wrought Iron 4'7" Length in inches beyond pin centre to form one eye equals 3.7 (P+R)		€5 8 8		23	24 25 25 27 26 27 27 27 27 28	3 2 2 2 3 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3	3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	36	35 feet.
cedua		76		216 2226 231	248 258 261 271	28 g 29 30 31	322 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	$35\frac{1}{2}$	Maximum shipping length should not exceed
one ey	BARS	H C		21 1 2 2 2 2 2 2 3 2 3 2 3 2 3 2 3 2 3 2	24 25 27 26 28	20 20 80 80 80 80 80 80 80 80 80 80 80 80 80	318 3218 3314 44 84	35	d not
form	OF	cs.		20 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	23.24.25.25.25.25.25.25.25.25.25.25.25.25.25.	27 ¹ / ₄ 28 ¹ / ₈ 29	03 14 20 00 00 00 00 00 00 00 00 00 00 00 00	348	nous
ron ntre to	R SIDE	$I_{\frac{7}{8}}$		19 3 20 1 2 2 1 1 2 2 2 1 1 8 2 2 1 1 1 8 2 2 1 1 1 1	23 24 24 25 25	20 27 2 2 2 2 2 3 3 3 3 3 3 3 3 3 3 3 3 3	301 321 321 331 441	$34\frac{1}{8}$	length
Wrought Iron 4'7"	DIAM. OR	$I^{\frac{3}{4}}$	18	1 8 8 1 2 0 8 8 2 2 0 8 8 8 8 8 8 8 8 8 8 8 8 8	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	264 274 288 29	30 31 31 ⁷ 32 ⁸ 32 ⁸	330	ipping
Wrong 4'7"	۵	$I^{\frac{5}{8}}$	171	1888 1988 201 201 211	22 23 24 24 24	25 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d 2 d	301 301 31 324	$33\frac{1}{4}$	um sh
\overrightarrow{y}		$I_{\frac{1}{2}}^{\frac{1}{2}}$	16 ¹ / ₈	1 8 1 1 8 4 4 2 0 5 4 4 5 6 5 6 4 5 6 5 6 5 6 6 6 6 6 6 6	222 88 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	25 8 26 4 27 1 28 8	29 30 31 31	324	Maxim
ni di		$I^{\frac{3}{8}}$	1 5 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	171 188 198 201 201	211 223 24	24 1 25 1 26 1 27 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	286 291 301 318 318	321	OTE:
Lengt		$I_{\frac{1}{4}}^{\frac{1}{4}}$	14 4 1 1 5 1 1 6 1 1 6 1 1 6 1 1 6 1 1 6 1 1 6 1 1 6 1 1 6 1 1 6 1 1 1 6 1	117 1188 1198 198	20 20 20 20 20 20 20 20 20 20 20 20 20 2	24 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	28 29 30 31	317	NC
		$I_{\overline{s}}^{\underline{t}}$	1 2 4 1 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	201 211 221 23	244 255 257 268 268 268	27 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	313	
		Ĭ	1 2 1 1 2 2 1 1 2 2 2 1 1 2 2 2 2 1 1 2	16 ¹ / ₈ 17 18 18 ⁷ / ₈	200 4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	23.4.4.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2	27 ¹ 28 ¹ 29 30	31	
		⊱ ∞	12 12 13 13 14 14 14	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	198 201 211 211 221	23 24 24 25 7 8	200 200 200 200 200 200 200 200 200 200	$30^{\frac{1}{2}}$	
d		00 74	112 122 133 144 144	15 ¹ 16 ¹ 17 17	1 8 1 9 4 1 1 0 1 1 0 1 1 1 1 1 1 1 1 1 1 1 1 1	22 23 24 25 25 8 8 8 8 8	200 200 200 200 200 200 200 200 200 200	30	
	DIAM.	PINS	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	क्षेत्र क्षेत्र क्षेत्र स्थान मान्नकान	02 02 02 02 HAMMANA	4444	50 5	9	

CLEVIS

All dimensions in inches,



Grip G can be made to suit connections.

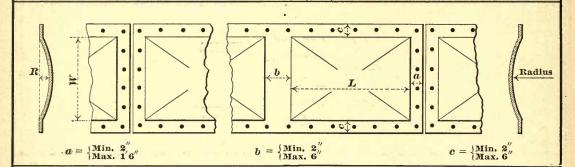
DIAM. OF				CLI	EVIS			DIAM. OF
CLEVIS ~	MAX, PIN	FORK	NUT	WIDTH	THICKNESS		-	CLEVIS
D	P	F	N	W	T	A	В	D
3	11/2	11/2	11/2	11/2	3 8	6	5	3
4	21/4	13/4	13/4	1.3	1/2	9	8	4
5	3	$2\frac{1}{4}$	21/4	24	<u>5</u> 8	9	8	5
6	31/2	23/4	23/4	234	34	9	8	6
7	4	31/4	34	31/4	7/8	9	8	7

Table giving diameter of Clevis for given rod and pin.

											-							
	ROD								PINS			1					ROD	
ROUND	SQUARE	UPSET	1	14	$1\frac{1}{2}$	13/4	2	$2\frac{1}{4}$	$2\frac{1}{2}$	$2\frac{3}{4}$	3	31/4	$3\frac{1}{2}$	34	4	UPSET	SQUARE	ROUND
34	5 8	1	3	3	3											1	5 .8	3/4
	3 4	118	3	3	3	4	4	4								11/8	3 4	
7 8	78	11/4		4	4	4	4	4								11/4	7 8	<u>7</u>
1		138		4	4	4	4	4								138		1
11/8	1	11/2		4	4	4	4	4	5	5	5					11/2	1	118
14	11/8	158			4	4	5	5	5	5	5					158	11/8	14
138		13/4			5	5	5	5	5	5	5					13/4		138
	11/4	178			5	5	5	5	5	5	5					17/8	14	
11/2	138	2				5	5	5	5	5	6	6	6			2	138	11/2
15 *		21/8				5	5	5	5	6	6	6	6			21/8		158
13/4	12	$2\frac{1}{4}$					6	6	6	6	6	6	7	7	7	$2\frac{1}{4}$	11/2	134
178	15	$2\frac{3}{8}$					6	6	6	6	7	7	7	7	7	23/8	15	178
2	13	$2\frac{1}{2}$						6	6	7	7	7	7	7	7	$2\frac{1}{2}$	13	2
21/8		25/8							7	7	7	7	7			25		21/8
	178	23/4							7	7	7	7	7			234	17/3	
21/4	2	27/8								7	7					27	2	21
ROUND	SQUARE	UPSET	1	11/4	11/2	13/4	2	$2\frac{1}{4}$	$2\frac{1}{2}$	$2\frac{3}{4}$	3	31/4	$3\frac{1}{2}$	$3\frac{3}{4}$	4	UPSET	SQUARE	ROUND
	ROD								PINS								ROD	

Clevises above and to right of heavy zigzag line, may be used with forks straight. Clevises below and to left of same line, should have forks closed in until pin is not overstrained.

BUCKLED PLATES.



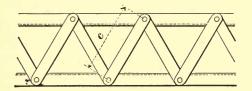
	OF	IN FEET A		RISE IN INCHES	IN FEET.A	BUCKLE ND INCHES	MAXIMUM NUMBER OF	No. OF	SIZE OF	ND INCHES	RISE	RAD. OF	ND INCHES	MAXIMUM NUMBER OF
	AIL	LENGTH .L	WIDTH	R	LENGTH L	WIDTH	BUCKLES	PLATE	WIDTH	LENGTH L	R	WIDTH	LENGTH	BUCKLES
	1	3 - 11	4-6	3 1/2	6-8	8-9	7	26	3 - 1	3-2	3	4-10	5 - 1	9
	2	4-6	3 -11	3 1/2	8-9	6-8	6	27	3-2	3 - 1	3	5 - 1	4-10	9
	3	3-11	3-6	3	7-9	6-3	7	28	3-1	3-0	3	4-10	4-7	9
	4	3-6	3 -11	3	6-3	7-9	8	29	3-0	3 - 1	3	4-7	4-10	9
	5	3-9	3-9	3	7-1	7-1	8	30	2-0	2-6	$2^{\frac{1}{2}}$	2-6	3-10	10
	6	3-1	3-9	3	4-10	7-1	9.	31	2-6	2-0	21/2	3 - 10	2-6	15
	7	3-9	3-1	3	7-1	4-10	8	32	3-6	5 - 6	$3\frac{1}{2}$	5 - 4	13 - 1	5
	8	3-8	3-8	2	10-2	10-2	8	33	5 - 6	3 - 6	$3\frac{1}{2}$	13 - 1	5 - 4	1
	9	2-8	3-8	2	5-5	10-2	10	34	4-0	4-0	3	8 - 1	8-1	7
3	10	3-8	2-8	2	10-2	5-5	8							
]	1	2-2	3-8	2	3-7	10-2	10					1		
1	12	3-8	2-2	2	10-2	3-7	8							
1	13	3-0	3-0	2	6-10	6-10	9							1-2
5	14	2-9	2-9	3	3-10	3-10	10							
2	19	2-6	2-9	$2\frac{1}{2}$	3-10	4-7	10						14 E III 1	
1	20	2-9	2-6	$2\frac{1}{2}$	4-7	3-10	10						H H	
1	21	2-6	2-6	$2\frac{1}{2}$	3-10	3-10	10							
6	22	3-5	3-6	3	5-11	6-3	8							
1	23	3-6	3 - 5	3	6-3	5 -11	8		-12-				4	
1	24	3-6	3-9	3 ·	6-3	7-1-	8				-4 -	1		
1	25	3-9	3-6	3	7-1	6-3	8							

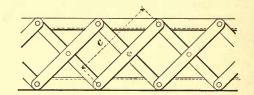
Plates are made $\frac{1}{4}$, $\frac{5''}{16}$, $\frac{8}{8}$ or $\frac{7''}{16}$ thick. Buckles of different sizes should not be used in the same plate. Rivets generally $\frac{5''}{8}$ or $\frac{8''}{4}$ diam.

FLOORING

CORRUGATED FLOORING	6	6 6 ¹ / ₁₆ 6 ¹ / ₈	14 4 5 16 3 8	$3 \\ 2\frac{31}{32} \\ 2\frac{16}{16}$	$1\frac{3}{6}$ $1\frac{26}{64}$ $1\frac{33}{32}$	6" 3 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2
	NOMINAL	неіднт И	THICKNESS	DISTANCE	GAUGE	STANDARD DIMENSIONS
	6"	3 8 7 16 1 2 9 18 8 8 8 11 11 16 8 4 4 13 16 7 8	3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6 3½ × 6	x 3 ½ x 3 ½	$\begin{array}{c} 1\frac{13}{16} \\ 1\frac{27}{52} \\ 1\frac{7}{6} \\ 1\frac{29}{32} \\ 1\frac{16}{16} \\ 1\frac{31}{32} \\ 2 \\ 2\frac{1}{39} \\ 2\frac{1}{16} \end{array}$	$3\frac{1}{4}$ $5\frac{3}{4}$ $3\frac{1}{4}$
Z BAR FLOORING	5"	5 10 3 8 7 10 12 2 9 10 10 11 10 5 8 8 11 10 10 3 4	$3\frac{3}{16} \times 5$ $3\frac{1}{4} \times 5$ $3\frac{1}{6} \times 5$ $3\frac{1}{16} \times 5$ $3\frac{1}{16} \times 5$ $3\frac{1}{16} \times 5$ $3\frac{3}{16} \times 5$ $3\frac{3}{16} \times 5$ $3\frac{1}{16} \times 5$ $3\frac{1}{16} \times 5$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$1\frac{25}{32}$ $1\frac{13}{16}$ $1\frac{27}{32}$ $1\frac{7}{8}$ $1\frac{29}{32}$ $1\frac{16}{16}$ $1\frac{31}{32}$ 2	$3\frac{1}{4}$
	4"	1 4 5 16 5 8 7 16 1 2 2 9 16 6 8 11 16 16 3 4	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c} 1\frac{3}{4} \\ 1\frac{3}{39} \\ 1\frac{18}{39} \\ 1\frac{18}{16} \\ 1\frac{27}{32} \\ 1\frac{7}{8} \\ 1\frac{29}{39} \\ 1\frac{15}{32} \\ 1\frac{31}{32} \\ 2 \end{array}$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
	NOMINAL	THICK	SIZE OF Z		gauge g	STANDARD DIMENSIONS

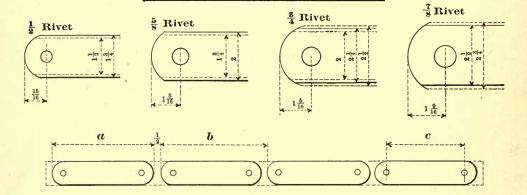
LACING





Maximum Distance c for given thickness of bar.

			-
SINGLE I	LACING $t-\frac{c}{40}$	DOUBLE LACI	NG $t - \frac{c}{co}$
THICK. $oldsymbol{t}$	DISTANCE . $oldsymbol{c}$	DISTANCE $oldsymbol{c}$	THICK. $oldsymbol{t}$
$\frac{1}{4}$,	0 - 10	1 - 3	1/4
<u>5</u> 16	1 - 0½	$1 - 6\frac{3}{4}$	1/4 5 16
3 8 7 16	1 - 3	$1 - 10^{\frac{1}{2}}$	3 8
7 16	1 - 51/2	$2 - 2\frac{1}{4}$	7 16
$\frac{1}{2}$	1 - 8	2 - 6	$\frac{1}{2}$
9 16	$1 - 10^{\frac{1}{2}}$	$2 - 9\frac{3}{4}$	9 16
<u>5</u> 8	2 - 1	$3 - 1^{\frac{1}{2}}$	<u>5</u> 8

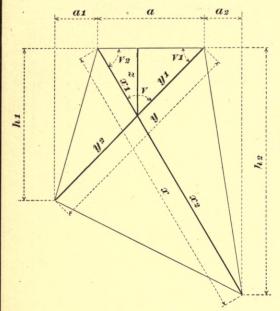


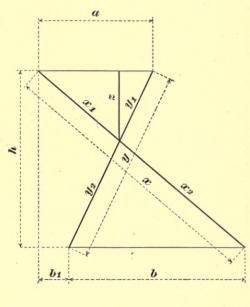
Distance to be added to C. C. length c.

WIDTH	FII	NISHED LEN	NGTH (a	01	RDERED LE	NGTH	b	WIDTH
OF		DIAM. O	FRIVET		2	DIAM. O	FRIVET		OF
BAR	1/2	· <u>5</u>	34	$\frac{7}{8}$	1/2	<u>5</u> 8	$\frac{3}{4}$	7/8	BAR
11/2	17/8				23/8			- 78	$1\frac{1}{2}$
13/4	17/8	$2\frac{3}{8}$			$2\frac{3}{8}$	$2\frac{7}{8}$	1		134
2		23/8	25/8			$2^{\frac{7}{8}}$.	$3\frac{1}{8}$		2
$2\frac{1}{4}$			25/8				31/8	4.	$2^{\frac{1}{4}}$
$2\frac{1}{2}$			$2\frac{5}{8}$	31/8			31	35	$2\frac{1}{2}$
23/4				$3\frac{1}{8}$		5	·	35	23/4
								1 .5 "	

All Dimensions in Inches

MENSURATION





$$x = \sqrt{(a+a_2)^2 + \overline{h_2}^2}$$

$$y = \sqrt{(a+a_1)^2 + \overline{h_2}^2}$$

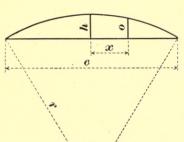
$$tan V_1 = \frac{h1}{a+a1}$$

$$tan V_2 = \frac{h2}{a+a2}$$

$$x_1 = \frac{a(\sin V_1)}{\sin V}$$

$$y_1 = \frac{a(\sin V_2)}{\sin V}$$

$$z = \frac{h \cdot 2 \cdot x_1}{x} = y^1(\sin V_1)$$



$$x = \sqrt{(b+b1)^2 + h^2}$$

$$x_1 = \frac{ax}{a+b}$$

$$y = \sqrt{(a-b1)^2 + h^2}$$

$$y^1 = \frac{ay}{a+b}$$

$$z - \frac{ah}{a+b}$$

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U. of C.
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Arc. = 00873 × diam. × ang. V

$$c = 2 \sqrt{r^2 - (r - h)^2}$$

$$r = \frac{c^2}{8h} + \frac{h}{2}$$

$$h = r - \sqrt{r^2 - \frac{c^2}{4}} = \frac{c^2}{8r} + \frac{h^2}{2r}$$

$$o = \sqrt{r^2 - x^2 - (r - h)}$$

WOOD SCREWS, SPIKES AND NAILS.

									CT	110100	CTEEL VA	UDE NAU				1
	OOD REWS			WROUGH ER TO A K							MON		HING	STEE	L WIRE S	SPIKES
NO.	DIAM.	LENGTH IN INCHES	1 INCH	5 16 INCH NO.	3 INCH	7 16 INCH NO.	1 INCH NO.	SIZE	HLENGTH	DIAM.	NO. PER POUND	DIAM. INCHES	NO. PER	LENGTH	DIAM. INCHES	NO. PER
0	.056	3	2250					2d	1	.0524	1060	.0453	1558	3	.1620	41
1	.069	31/2	1890	1208				3d	11/4	.0588	640	.0508	913	$3\frac{1}{2}$.1819	30
2	.082	4	1650	1135	*		-	4d	11/2	.0720	380	.0508	761	4	.2043	23
3	.096	$4\frac{1}{2}$	1464	1064			R	5d	13/4	.0764	275	.0571	500	$4\frac{1}{2}$.2294	17
4	.109	5	1380	930	742		-	6d	2	.0808	210	.0641	350	5	.2576	13
5	.122	6	1292	868	570			7d	$2\frac{1}{4}$.0858	160	.0641	315	$5\frac{1}{2}$.2893	11
6	.135	7	1161	662	482	445	306	Sd	$2\frac{1}{2}$.0935	115	.0720	214	6	.2893	10
7	.149	8		635	455	384	256	9d	$2\frac{3}{4}$.0963	93	.0720	195	$6\frac{1}{2}$.2249	71/2
8	.162	9	0 de	573	424	300	240	10d	3	.1082	77	.0808	137	7	.2249	7
9	.175	10			391	270	222	12d	31/4	.1144	60	.0808	127	8	.3648	5
10	.188	11			-20	249	203	16 d	$3\frac{1}{2}$.1285	48	.0907	90	9	.3648	$4\frac{1}{2}$
11	.201	12	. = 3.			236	180	20d	4	.1620	31	.1019	62			
12	.215							30d	$4\frac{1}{2}$.1819	22					
13	.228			101				4 0d	5	.2043	17		515		- 5	
14	.241							50d	$5\frac{1}{2}$.2294	13					
15	.255							60d	6	.2576	11					
16	.268							-		= =			(
17	.281		-						Terro							
18	.293														B	1
19	.308											18 18				
20	.321						4									
21	.334						3	14.14				1				
22	.347															
23	.361	U II					8									
24	.374	181								72						
25	.387	(3.)		1												
26	.401										V					
27	.414					2										
28	.427							131								
29	.440															124
30	.453				18											
-	1			-				10	-				1			

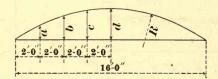
WROUGHT IRON TUBES

								н	DRAULI	C TUBIN	G.
- 4		ORDINA	RY GAS	OR WAT	ER PIPE			EX.	TRA	DOU EXT	BLE
NOMINAL DIAM.	OUTSIDE DIAM.	THICKNESS	INSIDE DIAM.	INTERNAL AREA	EXTERNAL AREA	WEIGHT PER. FOOT	THREADS PER. INCH	THICKNESS	INSIDE DIAM.	THICKNESS	INSIDE DIAM.
1/8	.40	.07	.27	.06	.13	.24	27	.10	.20		
$\frac{1}{4}$.54	.09	.36	.10	.23	.42	18	.12	.29		
3 8	.67	.09	.49	.19	.36	.56	18	.13	.42	.22	.23
1/2	.84	.11	.62	.30	.55	.84	14	.15	.54	.29	.24
34	1.05	.11	.82	.53	.87	1.12	14	.16	.73	.31	.42
1	1.31	.13	1.05	.86	1.36	167	11 ½	.18	.95	.36	.58
11/4	1.66	.14	1.38	1.49	2.15	2.24	11 1/2	.19	1.27	.38	.88
1 1/2	1.90	.15	1.61	2.03	2.84	2.68	11 1/2	.20	1.49	.40	1.08
2	2.37	.15	2.07	3.35	4.48	3.61	11 ½	.22	1.93	.44	149
$2\frac{1}{2}$	2.87	.20	2.47	4.78	6.49	5.74	8	.28	2.31	.56	1.75
3	3.50	.22	3.07	7.38	9,62	7.54	8	.30	2.89	.60	2.28
$3\frac{1}{2}$	4.00	.23	3.55	9.89	12.57	9.00	8	.32	3.35	.64	2.71
4	4.50	.24	4.03	12.73	15.90	10.66	8	.34	3.81	.68	3.13
41/2	5.00	.25	4.51	15.96	19.64	12.34	8	.35	4.25	.72	3.56
5	5.56	.26	5.05	19.99	24.30	14.50	8	.37	4.81	.75	4.06
6	6.63	.28	6.07	28.88	34.47	18.76	8	.44	5.75	.87	4.87
7	7.63	.30	7.02	38.73	45.66	23.27	8	.50	6.62	.84	6,06
8	8.63	.32	7.98	50.03	58 43	28.18	8	.56	7.50	.87	6.87
9	9.63	.34	8.94	62.73	73.72	33.70	8				
10	10.75	.36	10.02	78.84	90.76	40.06	8	Ш			-
12	12.75	.38	12.00	113.09		49.00	8				
13	14.00	.38	13.25			53.92	8				
14	15.00	.38	14.25			57.89	8				

Note: Above 15 inches the outside diameters are the nominal size.

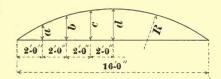
All dimensions given in inches, all weights in pounds.

ORDINATES for 16 foot chords



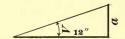
R	ADIUS	ORDIN	ATES FOR	16-0 TE	MPLET	RADIUS	ORDIN	ATES FO	R 16-0"TE	MPLET	RADIUS	ORDIN	ATES FOR	R 16-0 TEM	MPLET
11	IN NCHES	а	b	c	d	IN	a	b	c	d	INCHES	a	b	c	d
Г	200	11532	18 23	23 3 3 3	24 9 18	290	7 8 32	12 11 32	$15\frac{11}{32}$	16 11 32	520	3 18	6 23 32	8 13 32	8 15 18
1	02	11	18 ½	$22\frac{27}{32}$. 24 9 32	93	7 7 32	$12\frac{7}{32}$	15 3 18	16 3 18	30	3 7 8	6 19 32	8 1/4	8 25 32
L	04	10 7/8	18 %	22 19 32	24	96	$7\frac{1}{8}$	$12\frac{3}{32}$	$15\frac{1}{32}$	16	40	3 25 32	6 1/2	8 3 3 2	8 5 8
	06	$10\frac{3}{4}$	18 1/18	$22\frac{11}{32}$	$23\frac{3}{4}$	300	$7\frac{1}{32}$	$11\frac{18}{16}$	14 13 18	15 26 32	550	$3\frac{23}{32}$	6 11 32	729	8 7 18
ŀ	08	$10\frac{5}{8}$	17 7 8	22 3 32	$23\frac{15}{32}$	05	6 - 7 8	$11\frac{23}{32}$	14 9 16	15 ½	60	$3\frac{21}{32}$	6 7 32	$7\frac{25}{32}$	8.9
1	210	$10^{\frac{1}{2}}$	$17\frac{21}{32}$	21-7	$23\frac{7}{32}$	10	6 3/4	$11\frac{1}{2}$	14 9 32	$15\frac{7}{32}$	70	3 19 32	6 ½	$7\frac{21}{32}$	8 8 32
	12	10 ³ / ₈	$17\frac{15}{32}$	21 8	23	15	$6\frac{21}{32}$	$11\frac{5}{18}$	$14\frac{1}{18}$	15	80	3 17 32	6 1 32	$7\frac{1}{2}$	8
	14	10 9 32	$17\frac{5}{18}$	$21\frac{13}{32}$	223	20	6 %	1118	$13\frac{27}{32}$	14 3	90	$3\frac{7}{16}$	5 29 32	7 3 8	7 7 8
	16	10 5 32	17-18	21 3 18	$22\frac{1}{2}$	25	6 7 16	10 18	13 8	14 1/2	600	$3\frac{13}{32}$	5 13 18	71/4	$7\frac{23}{32}$
	18	1018	16 16	$20\frac{21}{32}$	22 32	30	$6\frac{11}{32}$	10 25	$13\frac{13}{32}$	14 0 32	10	3 11 32	5 23 32	7-18	7 19 32
1	220	9 18	16 ³ / ₄	20 3/4	$22\frac{1}{16}$	35	6 7 32	10 32	$13\frac{3}{16}$	14 18	20	3 9 32	5 8	$7\frac{1}{32}$	$7\frac{1}{2}$
	22	9 27 32	$16\frac{19}{32}$	2017	$21\frac{27}{32}$	40	6 32	107	13	13 32	30	3 1/4	5 17 32	$6\frac{29}{32}$	73/8
	24	9 3 4	16 13 32	$20\frac{11}{32}$	21 5 8	45	6 1/32	10 0 32	$12\frac{13}{16}$	13 8	40	3 3 16	5 7 10	635	71/4
-	26	9 5 8	161/4	20 1/8	$21\frac{13}{32}$	350	5 15 15	10 ½	12 19 52	13 7 16	650	3 1/8	5 11 32	611/16	7-1/8
ı	28	9 17 32	$16\frac{3}{32}$	$19\frac{15}{16}$	$21\frac{7}{32}$	55	$5\frac{27}{32}$	9 31 32	$12\frac{7}{16}$	13 7 32	60	3 32	5 9 32	6 19 32	7 1 32
1	230	9 7 18	15 15 18	19 32	21	60	5 3/4	9 13 16	$12\frac{1}{4}$	$13\frac{1}{32}$	70	3 1/32	5 3 16	61/2	6 32
1	32	911 32	$15\frac{25}{32}$	19 9 18	20 13	65	5 11/16	9 11 18	$12\frac{1}{18}$	$12\frac{7}{8}$	80	3	5 1/8	6 3 3 2	6 13
	34	9 82	15 5	19 3/8	$20\frac{19}{32}$	70	5 5 8	9 16	$11\frac{29}{32}$	1211	90	$2\frac{31}{32}$	5 1/16	6 8 15	$6\frac{23}{32}$
	36	9 5 32	15 15 32	19 3 18	$20\frac{13}{32}$	75	5 17 32	9 13 32	113/4	$12\frac{1}{2}$	700	$2\frac{29}{32}$	5	6 7/32	6 8
	38	91/18	15 11 32	$19\frac{1}{32}$	$20\frac{7}{32}$	80	5 7/18	9 9 32	11%	12 5	10	$2\frac{27}{32}$	4 7 8	63/32	61/2
1	240	831 32	15 3 18	1827	20132	85	5 8	9 5 32	$11\frac{13}{32}$	$12\frac{5}{32}$	20	$2\frac{13}{18}$	4 13 18	61/32	67/18
ı	43	8 7/8	15	18 19 32	19 32	90	5 18	91/32	119	12	30	$2\frac{26}{32}$	4 3 4	5 15 15	6 11 32
1	46	8 3/4	14 25 32	$18\frac{11}{32}$	19 1/2	95	5 1/4	8 15 16	11½	$11\frac{27}{32}$	40	2 3/4	4 11 18	5 7 8	61/4
1	250	8 19 32	14 17 32	$18\frac{1}{32}$	$19\frac{3}{18}$	400	5 5 32	8 13 16	$10\frac{31}{32}$	11118	750	$2\frac{23}{32}$	4 32	5 32	6 3
	53	8 18 32	14 11 32	$17\frac{25}{32}$	18 18	10	5 1 32	8 19 32	10 33	11 13 32	60	$2\frac{21}{32}$	4 9 18	5 28 32	632
1	56	8 11 32	14 5 32	17 18	18 11 10	20	$4\frac{29}{32}$	8 3 8	10 7/18	11 1 8	70	2 8	4 1/2	5 8	6
1	260	8 7 32	13 29 32	17 32	18 ³ / ₈	30	4 28 32	8 3 10	10 3/16	10 7/8	so	2 19 32	4 7/16	5 18	5 18
ı	63	81/8	13 3/4	1718	18 5 32	40.	4 21 32	$7\frac{31}{32}$	9 15 18	$10\frac{19}{32}$	90	$2\frac{19}{32}$	4 13 32	5 ½	5 7/8
L	66	8	13 %	$16\frac{27}{32}$	$17\frac{27}{32}$	450	4 9 18	7 13	9 28 32	10 3 8	800	2 17 32	4 11 32	5 7/18	5 25 32
1	270	7 7 8	13 11 32	16 ⁵ / ₈	1731	60	$4^{\frac{16}{32}}$	7 5 8	9 1/2	10 1/8	10	$2^{\frac{1}{2}}$	4 32	5 11/32	5 33
	73	$7\frac{25}{32}$	13 3 18	16 ³	177	70	4 3 8	7 15 32	9 5 16	9 29 32	20	$2\frac{15}{32}$	4 1/4	5 9 9 9	5 21 32
	76	$7\frac{11}{18}$	$13\frac{1}{32}$	163	$17\frac{1}{4}$	80	4 1/4	7 32	9 3 3 2	9 11 18	30	$2\frac{15}{32}$	4 3 16	5 4	5 19 32
1	280	7 0 16	12 32	1515	1631	90	4 3 15	7 5 32	8 29 32	91/2	40	$2\frac{19}{32}$	41/8	5 32	5 1/2
	83	$7\frac{15}{32}$	12 11/15	15 32	$16\frac{25}{32}$	500	$4\frac{3}{32}$	7	8 33	9 8 18	850	2 3 8	$4\frac{3}{32}$	5 3 3 2	5 7 16
L	86	7 12	12 17 32	$15\frac{10}{32}$	$16\frac{19}{32}$	10	4 1/32	6 7 8	8 %	9 1/6	60	2 3 8	$4^{\frac{1}{32}}$	5 1/18	5 3/8

ORDINATES for 16 foot chosds



RADIUS	ORDIN	ATES FO	R 16-0"TE	MPLET	RADIUS	ORDIN	ATES FOR	16-0"TE	MPLET	RADIUS	ORDIN	ATES FOR	16-0 TEM	MPLET
INCHES	a	b	c	đ	INCHES	а	b	c	d	INCHES	a	b	c	d
870	$2\frac{11}{32}$	4	5	5 5 16	1460	1 38	238	231 32	3 5 3 3	2900	11 16	1 3 16	1 15 32	1 0
80	2 8	3 10	4 15	5-1/4	90	1 3	$2\frac{5}{16}$	$2\frac{29}{32}$	3 33	50	11 16	1 32	1 7 16	1 10
90	2 32	3 33	4 7 8	5 3/16	1520	1 32	2 🕯	237	3 1/32	3000	11 16	132	1 7 10	1 3 3
900	2 1/4	3 32	$4^{\frac{13}{16}}$	5 1/8	50	1 8	$2\frac{7}{32}$	$2\frac{2\delta}{32}$	$2\frac{31}{32}$	3100	21 32	1 1	1 33	$1^{\frac{1}{2}}$
10	2 32	3 18	4 3	5 18	80	$1\frac{1}{4}$	$2\frac{3}{16}$	233	2 20 33	3200	<u>8</u>	1 3 3 3	1 11 32	17/16
20	2 3 16	3 28	423	5 1/3 2	1620	$1\frac{7}{32}$	$2\frac{5}{32}$	$2\frac{11}{16}$	$2\frac{27}{32}$	3300	19 33	1 1 16	1 5	1 13 33
30	$2\frac{3}{10}$	3 3	4 32	$4\frac{31}{32}$	50	$1\frac{7}{32}$	$2\frac{3}{32}$	25/8	$2\frac{25}{34}$	3400	9 16	139	$1\frac{1}{4}$	1 11 32
40	2 5 32	$3\frac{23}{32}$	4 ⁵ / ₈	4 15	80	1 3	$2\frac{1}{16}$	2 %	2 3	3500	16	1	1 1/4	1 18
950	$2\frac{5}{32}$	3 11 16	4 19 32	4 7 8	1710	$1\frac{3}{16}$	$2\frac{1}{33}$	2 17 32	211/16	3600	17 32	31 33	1 7 3 3	1 9 33
60	2 1/8	3 8	$4\frac{17}{32}$	$4\frac{37}{32}$	40	1 32	2	$2\frac{15}{33}$	$2\frac{21}{33}$	3700	17 33	15 16	1 3	1 1/4
70	2 1/8	3 8	4 ½	$4^{\frac{25}{33}}$	70	1 1/8	1 15	27/16	$2\frac{19}{32}$	3800	17	29 33	1 5 3 3	1 7/32
80	$2\frac{1}{10}$	3 %	4 7 16	$4\frac{23}{32}$	1800	$1\frac{1}{8}$	1 29 33	$2\frac{13}{32}$	2 %	3900	17 32	39 33	1 1/8	1 3 10
90	$2\frac{1}{32}$	3 1/3	4 ³ / ₈	4 21 32	30	$1\frac{3}{32}$	1 7 8	2 11/33	2 ½	4000	1/2	78	1 32	133
1000	$2\frac{1}{32}$	3 15 32	$4\frac{11}{32}$	4 ⁸	60	$1\frac{3}{32}$	$1\frac{27}{32}$	2 8	2 18 33	4100	1/2	27 32	1 1 16	1 1 8
20	2	3 13 3	41/4	4 17 33	90	$1\frac{1}{16}$	1 27 32	2 %	$2\frac{7}{16}$	4200	18 33	13 16	1 1 3 3	133
40	1 15	3 11 32	4 33	4 7/16	1920	$1\frac{1}{10}$	1 13	2 1/4	2 13 32	4300	15 32	13 16	1	1 10
60	1 32	3 33	433	4 8	50	1 10	1 33	$2^{\frac{1}{4}}$	$2\frac{3}{8}$	4400	15 32	13 16	31	1 10
80	1 39	3 7/32	4 1 33	4 32	2000	1	1 23 33	2 8 32	$2\frac{5}{16}$	4600	7	35 32	15 16	1
1100	1 3 3	3 32	3 15 16	4 7 32	50	1	1 11 15	$2\frac{1}{6}$	$2\frac{1}{4}$	4800	7 16	23	33	31 32
20	1 13 16	3 3 3 3	3 7 8	418	2100	31 32	1 21/32	21/16	$2\frac{7}{32}$	5000	13	23 32	78	1 <u>6</u>
40	1 3 3 2	3 1/32	3 13 16	$4\frac{1}{16}$	50	33	1 32	$2\frac{1}{32}$	$2\frac{5}{32}$	5200	13	21 32	32	29 32
60	1 3	$2^{\frac{31}{32}}$	3 33	3 31/32	2200	1 <u>5</u>	1 32	2	2 1/8	5400	3	31 32	13	7 8
80	1 33	$2^{\frac{15}{16}}$	3 31 32	3 33	50	39 32	1 16	1 15	$2\frac{1}{16}$	5600	3 8	31 32	13 16	37 32
1200	1 11	$2\frac{7}{8}$	3 32	3 27 33	2300	33	1 17 33	1 32	2 1 32	5800	3 B	32	3 4	13 16
20	1-21	$2^{\frac{27}{32}}$	3 %	$3\frac{25}{32}$	50	78	1 1/2	$1\frac{27}{32}$	1 31 32	6000	11 32	19 32	3 4	33
40	1 ੈ	$2\frac{13}{16}$	3 1/2	3 33	2400	27 32	1 7/16	1 13	1 16	6150	32	9 16	23 33	3 4
60	1 19 32	2 3/4	3 7	3 21 32	50	13 16	1 13 32	1 3/4	1 7/8	6350	3 16	9	11 16	3.
80	1 16	2 33	3 3 8	3 32	2500	13 16	1 8	1 33	1 37 32	6500	5 16	17 32	21 32	23 33
1300	1 17/32	$2\frac{21}{32}$	3 5 16	3 17 32	50	28 32	1 11/32	1 11/16	1 13 16	7000	32	13	8	21 33
20	1 1/2	$2\frac{19}{32}$	3 4	3 33	2600	25 32	1 11/32	$1\frac{21}{32}$	1 33	7500	32	15 32	19 32	5 8
40	1 13	2 %	3 3	3 13 33	50	34	1 3 16	1 5	1 3/4	8000	1/4	7 16	17 33	16
60	1 15 32	2 17	3 5 32	3 3	2700	3	1 33	$1\frac{19}{32}$	123	8500	1/4	13 32	1 2	9 16
80	1 7/16	2 1/2	3 1/8	3 11 32	50	3 4	1 1/4	1 10	1 11 10	9000	32	3/8	15 32	1/2
1400	1 7 10	2 15	3 32	3 5 16	2800	33	1 7/32	$1\frac{17}{32}$	1 32	10000	33	11 32	7 18	10 32
30	1 13 32	$2\frac{13}{32}$	3 - 32	3 7 32	50	23 32	1 33	1 ½	1 5					

Table of Bevels



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L	DIST	a	0	3,2	19	sol _{G0}	⇔ ∞	900	100	528	-14	0 100	10	11	യയ	32	16	in (c)	⊷lca	17	100	300	හ ග	100	= 10	80 80	ω 4 ₁	8 8 8	13	982	⊳ ∞	CI (CI	100	2000
11	-В .	NIN.	31	36	40	45	20	55	8	00	10	14	19	24	28	33	38	42	47	51	56	10	90	9	15	20	24	29	33	38	43	47	51	28
1	ANGLE	DEG.	42	42	42	42	42	42	43	43	43	43	43	43	43	43	43	43	43	43	43	44	44	44	44	44	44	44	44	44	44	44	44	44
) " (ы ,	MIN.	48	54	59	02	10	15	20	25	30	35	40	46	51	56	01	90	11	16	21	26	31	37	41	46	51	99	01	90	11	16	21	26
10	ANGLE	DEG.	39	39	39	40	40	40	40	40	40	40	40	40	40	40	4	4	4	4	4	41	41	41	4	41	41	4	42	42	43	42	42	42
"	E <.	MIN.	52	58	40	10	15	20	26	32	38	43	49	55	8	05	11	16	22	27	33	39	44	20	55	8	90	Ħ	16	21	27	32	38	43
6	ANGLE	DEG.	36	36	37	37	37	37	37	37	37	37	37	37	38	38	38	38	38	38	38	38	38	38	38	39	39	39	39	39	39	39	39	39
"	E <.	MIN.	42	48	54	8	07	13	18	24	30	37	43	49	55	8	0.7	13	19	25	31	36	42	48	55	8	90	12	17	24	30	35	40	46
8	ANGLE	DEG.	33	33	33	34	34	34	34	34	34	34	34	34	34	35	35	35	35	35	35	35	35	35	35	36	36	36	36	36	36	36	36	36
,	>	NIN.	15	22	29	35	42	49	99	02	60	15	22	28	35	4	48	54	00	07	14	20	26	32	38	45	51	28	40	9	17	23	29	35
112	ANGLE	DEG.	30	30	30	30	30	30	30	31	31	31	31	31	31	31	31	31	32	32	32	32	32	32	32	32	32	32	33	33	33	33	33	33
"	>	MIN.	34	4	49	55	03	10	17	24	30	38	45	52	58	90	13	20	27	34	41	48	55	02	80	15	22	28	35	42	48	55	02	60
9	ANGLE	DEG. N		26		26	27	27		27			27		27	28	28		28		_	28	28	29	_	29		29	29	29			30	-
	>	MIN.	37	45	53	8	80	15	23	30	38	46	53	8	80	16	23	90	38	45		8	80		22	29		44	-	28			20	
20.	ANGLE	DEG. N		_	22		23 (23		23					24		-	24	24		25 (25				_	25		-	26	
	>	MIN.		35	_	20 3	59			23	30		_	54			18			41 2	49		05		20		35	44	52	_			22	-
4"	ANGLE	DEG. M		18		18	18			19	19		19 4	19	20		20				20 4			21	_	-		_	21 5				22	
	>	MIN.				28	36		53	70	60	_	26	34	42	51 2	00	80		24	-	40	49	57 2	05 2	13	21	30 2	38	46	54 2	-	10	
33	ANGLE	DEG. M		14		14 5	14 3	14 4		15 0		15 1	_			15 5		16 0	16 1	16 2	16 3		16 4	-	17 0		17 2	17 3		17 4	17 5		18 1	
	, A	MIN.			_				_	29 1	38 1		55 1	_	-	_				-	-	-	-						-	-				
2	ANGLE						_					_	_	_			1 29			_	2 03				_				3 12		13 2		3 45	_
-	A	N. DEG	_				_		_							_	_			_						_	-	-	-			-		
1	ANGLE	DEG. MIN.			-						5 57	_					_									_	_	_			53			
-			4			_			_	_						_		_	_	_			-		_	•		-	-		- 8	Н		
0	ANGLE V.	MIN.		-			36			_				38		_	90			_		-			-						10	-		
L	Ļ	DEG		_		_		_	-		_			_						-									_		4		-	_
L	DIST	a	°	-18	19	E 60	→ ∞	10 00	100	P 20	← 4	9 0	16	11 8	ധിത	5100	191	1000	⊷lcd	1700	0 0	323	ကြေ	Calon Calon	110	20 00	w 4	3000	101	0 0	⊳ ∞	000	118	32

NATURAL TANGENTS.

-														0
DE-	o'	5	10'	15	20	25	30'	35	40	45	50	55	60'	DE- GREE8
0	.0000	.0015	.0029	.0044	.0058	.0073	.0087	.0102	.0116	.0131	.0146	.0160	.0175	0
1	.0175	.0189	.0204	.0218	.0233	.0247	.0262	.0276	.0291	.0306	.0320	.0335	.0349	1
2	.0349	.0364	.0378	.0393	.0407	.0422	.0437	.0451	.0466	.0480	.0495	.0509	.0524	2
3	.0524	.0539	.0553	.0568	.0582	.0597	.0612	.0626	.0641	.0655	.0670	.0685	.0699	3
4	.0699	.0714	.0729	.0743	.0758	.0772	.0787	.0802	.0816	.0831	.0846	.0860	.0875	4
5	.0875	.0890	.0904	.0919	.0934	.0948	.0963	.0978	.0992	.1007	.1022	.1036	.1051	5
6	.1051	.1066	.1080	.1095	.1110	.1125	.1139	.1154	.1169	.1184	.1198	.1213	.1228	6
7	.1228	.1243	.1257	.1272	.1287	.1302	.1317	.1331	.1346	.1361	.1376	.1391	.1405	7
8	.1405	.1420	.1435	.1450	.1465	.1480	.1495	.1509	.1524	.1539	.1554	.1569	.1584	8
9	.1584	.1599	.1614	.1629	.1644	.1658	.1673	.1688	.1703	.1718	.1733	.1748	.1763	9
10	.1763	.1778	.1793	.1808	.1823	.1838	.1853	.1868	.1883	.1899	.1914	.1929	.1944	10
11	.1944	.1959	.1974	.1989	.2004	.2019	.2035	.2050	.2065	.2080	.2095	.2110	.2126	11
12	.2126	.2141	.2156	.2171	.2186	.2202	.2217	.2232	.2247	.2263	.2278	.2293	.2309	12
13	.2309	.2324	.2339	.2355	.2370	.2385	.2401	.2416	.2432	.2447	.2462	.2478	.2493	13
14	.2493	.2509	.2524	.2540	.2555	.2571	.2586	.2602	.2617	.2633	.2648	.2664	.2679	14
15	.2679	.2695	.2711	.2726	.2742	.2758	.2773	.2789	.2805	.2820	.2836	.2852	.2867	15
16	.2867	.2883	.2899	.2915	.2931	.2946	.2962	.2978	.2994	.3010	.3026	.3041	.3057	16
17	.3057	.3073	.3089	.3105	.3121	.3137	.3153	.3169	.3185	.3201	.3217	.3233	.3249	17
18	.3249	.3265	.3281	.3298	.3314	.3330	.3346	.3362	.3378	.3395	.3411	.3427	.3443	18
19	.3443	.3460	.3476	.3492	.3508	.3525	.3541	.3558	.3574	.3590	.3607	.3623	.3640	19
20	.3640	.3656	.3673	.3689	.3706	.3722	.3739	.3755	.3772	.3789	.3805	.3822	.3839	20
21	.3839	.3855	.3872	.3889	.3906	.3922	.3939	.3956	.3973	.3990	.4006	.4023	.4040	21
22	.4040	.4057	.4074	.4091	.4108	.4125	.4142	.4159	.4176	.4193	.4210	.4228	.4245	22
23	.4245	.4262	.4279	.4296	.4314	.4331	.4348	.4365	.4383	.4400	.4417	.4435	.4452	23
24	.4452	.4470	.4487	.4505	.4522	.4540	.4557	.4575	.4592	.4610	.4628	.4645	.4663	24
25 26	.4663	.4681	.4699	.4716	.4734	.4752	.4770	.4788	.4806	.4823	.4841	.4859	.4877	25
27	.4877	.4895	.4913	.4931	.4950	.4968	.4986	.5004	.5022	.5040	.5059	.5077	.5095	26 27
28	.5095	.5114	.5132	.5150	.5169	.5187	.5206	.5224	.5243	.5261	.5280	.5298	.5317	28
29	.5543	.5562	.5354	.5600	.5392	.5639	.5430	.5448	.5467	.5486	.5735	.5754	.5774	29
30	.5774	.5793			.5851							_	.6009	30
31	.6009	.6028	.5812	.5832	.6088	.5871	.5890	.5910	.5930	.5949	.5969	.5989	.6249	31
32	.6249	.6269	.6289	.6310	.6330	.6350	.6128	.6391	.6412	.6432	.6453	.6473	.6494	32
33	.6494	.6515	.6536	.6556	.6577	.6598	.6619	.6640	.6661	.6682	.6703	.6724	.6745	33
34	.6745	.6766	.6787	.6809	.6830	.6851	.6873	.6894	.6916	.6937	.6959	.6980	.7002	34
35	.7002	.7024	.7046	.7067	.7089	.7111	.7133	.7155	.7177	.7199	.7221	.7243	.7265	35
36	.7265	.7288	.7310	.7332	.7355	.7377	.7400	.7422		.7467	.7490	.7513	.7536	
37	.7536	.7558	.7581	.7604	.7627	.7650	.7673	.7696	.7720	.7743	.7766	.7789	.7813	37
38	.7813	.7836	.7860	.7883	.7907	.7931	.7954	.7978	.8002	.8026	.8050	.8074		38
39	.8098	.8122	.8146	.8170	.8195	.8219	.8243	.8268	.8292	.8317	.8342	.8366	.8391	39
40	.8391	.8416	.8441	.8466	.8491	.8516	.8541	.8566	.8591	.8617	.8642	.8667	.8693	40
41	.8693	.8718	.8744	.8770	.8796	.8821	.8847	.8873	.8899	.8925	.8952	.8978	.9004	41
42	.9004	.9030	.9057	.9083	.9110	.9137	.9163	.9190	.9217	.9244	.9271	.9298	.9325	42
43	.9325	.9352	.9380	.9407	.9435	.9462	.9490	.9517	.9545	.9573	.9601	.9629	.9657	43
44	.9657	.9685	.9713	.9742	.9770	.9798	.9827	.9856	.9884	.9913	.9942	.9971	1.0000	44
DE- GREES	O'	5'	10'	15'	20'	25'	3.0'	35'	40'	45	50'	55'	60'	DE- GREES
							44							0

MULTIPLICATION TABLE

For Rivetspacing

SPACES							PITC	H IN IN	CHES							SPACES
SPA	13	11/4	13/8	$1\frac{1}{2}$	15/8	$1\frac{3}{4}$	17/8	2	21/8	$2\frac{1}{4}$	$2\frac{3}{8}$	$2\frac{1}{2}$	$2rac{5}{8}$	$2\frac{3}{4}$	2 7/8	SPA
1																1
2	- 21/4	- 21/2	- 23	- 3	- 31/4	- 31/2	- 33	- 4	- 41/4	- 41/2	- 43	- 5	- 51/4	- 5½	- 54	2
3	- 38	- 33	- 418	- 41	- 47	-, 5 4	- 55	- 6	- 68	- 63	- 7 ¹ / ₈	- 71/2	- 778	- 81/4	- 8 ₈	3
4	- 41/2	- 5	- 5½	- 6	- 6½	- 7	- 7½	- 8	- 8½	- 9	- 9½	- 10	-10½	-11	-1112	4
5	- 55	- 6 ¹ / ₄	- 6 ⁷ 8	- 71	- 8 ¹	- 8 ³ / ₄	- 9 ³ / ₈	-10	-10 ⁵	-11 ¹ / ₄	-11 ⁷ / ₈	1- 01/2	1- 1 ¹ / ₈	1- 13/4	1- 23	5
6	- 63	- 7½	- 8 ¹ / ₄	- 9	- 93	-1 0½	-11 ₄	1-0	1- 03	1- 11/2	1- 21/4	1- 3	1- 33	1- 41/2	1- 51/4	6
7	- 778	- 8 ³	- 95	-10½	-11 ³ / ₈	1- O ¹ / ₄	1- 11	1-2	1- 27/8	1- 33/4	1- 45	1- 51/2	1- 68	1-74	1-81	7
8	- 9	-1 0	-11	1-0	1- 1	1-2	1-3	1-4	1-5	1-6	1-7	1-8	1-9	1-10	1-11	8
9	-10 ¹ / ₈	-11 ¹ ₄	1- 08	1- 11/2	1- 25	1- 3\frac{3}{4}	1- 47/8	1-6	1- 71/8	1-81/4	1- 98	1-101/2	1-115	2- 03/4	2- 17/8	9
10	-1114	1 01/2	1- 13/4	1-3	1- 41	1- 5\frac{1}{2}	1- 63/4	1-8	1- 91/4	1-10½	1-113/4	2-1	2- 21/4	2- 31/2	2- 43/4	10
11	1- 0 ³	1- 13/4	1- 3½	1- 41/2	1- 5%	$1 - 7\frac{1}{4}$	1- 85	1-10	1-118	2- 03/4	2- 2½	2- 31/2	2- 47/8	2- 64	2- 7 8	11
12	1- 11/2	1-3	1- 41/2	1-6	1- 71/2	1-9	1-101	2-0	2- 11/2	2-3	2- 41/2	2-6	$2-7\frac{1}{2}$	2-9	$2-10\frac{1}{2}$	12
13	1- 25/8	1- 41/4	1- 5 ⁷ 8	1- 71/2	1- 9 ¹ / ₈	1-10 3	2- 03	2- 2	2- 3 5 8	2- 51/4	2- 6 ⁷ 8	2- 81/2	2-10 ¹ / ₈	2-11 ³ / ₄	3- 1 ³ / ₈	13
14	1- 33	1- 51/2	1- 74	1-9	1-103	2- 0½	2- 21/4	2-4	$2-5\frac{3}{4}$	2- 71/2	2- 91/4	2-11	$3-0\frac{3}{4}$	$3-2\frac{1}{2}$	3- 41/4	14
15	1- 47 a	1- 63	1- 85	1-102	2- 0 ³	2- 21/4	2- 41/8	2-6	2- 778	2- 93/4	$2-11\frac{5}{8}$	3- 11/2	3- 38	3- 51/4	3- 71	15
16	1- 6	1-8	1-10	2-0	2- 2	2-4	2-6	2-8	2-10	3-0	3- 2	3-4	3- 6	3-8	3-10	16
17	1- 718	1- 91/4	1-113g	2- 1 ¹ / ₂	2- 35	$2 - 5\frac{3}{4}$	2- 7-8	2-10	3- 0 ¹ / ₈	3- 21/4	3- 43	3- 6½	3- 85	3-104	4- 07	17
18	1-81	1-101	2- 03/4	2- 3	2- 51/4	2- 71/2	2- 93/4	3- 0	3- 21/4	3- 41/2	3- 63	3-9	3-1114	4- 11/2	4- 34	18
19	1- 9 ³ ₆	1-113	2- 21/8	2- 41/2	2- 67	2- 91/4	2-11 8	3- 2	3- 43	3- 63/4	3- 91	$3-11\frac{1}{2}$	4- 17/8	4- 41/4	4- 6	19
20	1-101	2- 1	2- 31/2	2- 6	2- 81	2-11	3- 11/2	3-4	3- 6½	3- 9	$3-11\frac{1}{2}$	4- 2	4- 41/2	4-7	4- 91/2	20
21	1-115	2- 21/4	2- 4 ⁷ ₈	2- 71/2	2-101	3- 03	3- 3 8	3-6	3- 85	3-1114	4- 1 ⁷ 8	4- 41/2	4- 718	4- 93	5- 0 ³ / ₈	21
22	2- 03	2- 3½	2- 64	2- 9	2-113/4	3- 21/2	3- 51/4	3-8	3-103	4- 11/2	4- 41/4	4-7	4- 93	5- O ¹ / ₂	5- 31/4	22
23	2- 17/8	2- 43/4	2- 75	2-101	3- 13	3- 41/4	3- 71/8	3-10	4- 0 ⁷ 8	4- 33/4	4- 6 ⁵	4- 91/2	5- 0 ³	·5- 3 ¹ / ₄	5- 6 ¹ / ₈	23
24	2- 3	2-6	2- 9	3- 0	3- 3	3- 6	3- 9	4-0	4-3	4-6	4-9	5-0	5- 3	5- 6	5- 9	24
25	2- 41	2- 74	2-10 ³	3- 11/2	3- 45	3- 73/4	3 -1 0%	4-2	4- 5½	4- 81/4	4-11 ³	5- 2½	5- 5 ⁵ 8	5-83	5-11 ⁷ / ₈	25
26	2- 51/4	2- 8½	2 - 11 ³ / ₄	3- 3	3- 61/4	3- 91/2	4- 0 3	4-4	4- 71/4	4-10½	5- 1 ³ / ₄	5- 5	5-81/4	5-11½	$6-2\frac{3}{4}$	26
27	2- 68	2- 93/4	3- 1 ¹ / ₈	3- 41/2	3- 77	3-1114	4- 25/8	4-6	4- 9 ³ / ₈	5- O ₄	5- 41/8	5- 71/2	5-10 ⁷ ₈	6- 21/4	6- 5 ⁵ / ₈	27
28	$2 - 7\frac{1}{2}$	2-11	3- 21/2	3- 6	3- 91/2	4- 1	4- 4½	4-8	4-11 ¹ / ₂	5-3	5- 6½	5-10	6- 11/2	6- 5	6- 8½	28
29	2- 85	3- O4	3- 37/8	3- 7½	3-11 ¹ / ₈	4- 23/4	4- 6 ³ / ₈	4-10	5- 1 ⁵ / ₈	5- 54	5- 8 ⁷ / ₈	6- 0½	6- 4½	6- 7\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	6-11 ³	29
30	2- 94	3- 1½	3- 514	3- 9	4- 03/4	4- 41/2	4- 81	5- 0	5- 3-3/4	5- 71/2	5-11 ¹ / ₄	6- 3	6- 63	6-10 ¹ / ₂	7- 21/4	30
SPACES	11/8	11/4	13/8	11/2	18	$1\frac{3}{4}$	17/8	2	21/8	$2\frac{1}{4}$	$2\frac{3}{8}$	$2\frac{1}{2}$	25/8	$2\frac{3}{4}$	27/8	SPACES
SPA							PITO	CH IN IN	CHES							SPA

MULTIPLICATION TABLE

For Rivetspacing

CES							PITO	сн ій ій	CHES							SES
SPACES	3	31/8	31/4	$3\frac{3}{8}$	$3\frac{1}{2}$	$3\frac{3}{4}$	4	41/4	41/2	$4\frac{3}{4}$	5	$5\frac{1}{4}$	$5\frac{1}{2}$	$5\frac{3}{4}$	6	SPACES
1																1
2	-6	- 61	-61/2	- 63	- 7	- 7½	-8	- 81	- 9	- 91	-10	-10 ¹ / ₃	-11	-11½	1-0	2
3	-9	- 98	- 9 3	-10 ½	-10 ½	- 11 ¹ / ₄	1-0	1- O ₄	1- 11/2	1- 24	1- 3	1- 34	1- 41	1- 51/4	1-6	3
4	1-0	1- 0½	1- 1	1- 11/2	1- 2	1-3	1-4	1- 5	1- 6	1- 7	1-8	1- 9	1-10	1-11	2-0	4
5	1-3	1- 35	1- 41/4	1- 47/8	1- 5½	1- 63/4	1-8	1- 91/4	1-101	1-113	2- 1	2- 21/4	2- 3	2- 43	2-6	5
6	1-6	1- 63	1- 71/2	1-81	1- 9	1-101	2-0	2- 11/2	2- 3	2- 41/2	2-6	2- 71	2- 9	2-101	3-0	6
7	1-9	1- 9-7	1-10 3/4	1-11-8	2- 01/2	2- 21	2-4	2- 5\frac{3}{4}	2- 71/2	2- 91/4	2-11	3- 0∄	3- 21/2	3- 44	3-6	7
8	2-0	2-1	2- 2	2-3	2-4	2- 6	2-8	2-10	3- 0	3- 2	3- 4	3- 6	3-8	3-10	4-0	8
9	2-3	2- 41/8	2- 51/4	2- 63	2- 71/2	2- 93/4	3-0	3- 21/4	3- 41/2	3- 64	3- 9	3-1114	4- 11/2	4- 3\\\\3\\\\4	4-6	9
10	2-6	2- 74	2-81/2	2- 93	2-11	3- 1 ¹ / ₃	3-4	3- 61/2	3- 9	3-111/2	4- 2	4- 41/2	4-7	4- 91	5-0	10
11	2-9	2-103	2-11 3	3- 1 ¹ / ₈	3- 2½	3- 5 1/4	3-8	$3-10\frac{3}{4}$	4- 1 ¹ / ₂	4- 41/4	4-7	4- 94	5- O1	5- 31	5-6	11
12	3-0	3-11/2	3- 3	3- 4½	3- 6	3- 9	4-0	4- 3	4-6	4- 9	5-0	5- 3	5- 6	5- 9	6-0	12
13	3-3	3- 45	3- 61/4	3- 77/8	3- 91/2	4- 03	4-4	4- 74	4-101	5- 1 ³ / ₄	5- 5	5- 81	5-11½	6- 23/4	6-6	13
14	3-6	3- 73	3- 91/2	3-111	4- 1	4- 41	4-8	4-1112	5-3	5- 61	5-10	6- 11/2	6- 5	6- 81/2	7-0	14
15	$3-9 3-10^{\frac{7}{8}} 4-0^{\frac{3}{4}} 4-2^{\frac{5}{8}} 4-4^{\frac{1}{2}} 4-8^{\frac{1}{4}} 5-0 5-3^{\frac{3}{4}} 5-7^{\frac{1}{2}} 5-11^{\frac{1}{4}} 6-3 6-6^{\frac{3}{4}} 6-10^{\frac{1}{2}} 7-2^{\frac{1}{4}} 7-6 1$															15
16	4-0 4-2 4-4 4-6 4-8 5-0 5-4 5-8 6-0 6-4 6-8 7-0 7-4 7-8 8-0 1															16
17	4-3	4- 5½	4- 71/4	4- 9를	4-1112	5- 33	5-8	6- 04	6- 41/2	6- 83	7- 1	7- 54	7- 91	8- 13	8-6	17
18	4-6	4-81/4	4-101	5- O ³ / ₄	5-3	5- 71/2	6-0	6- 4½	6-9	7- 11/2	7- 6	7-101	8- 3	8- 71/2	9-0	18
19	4-9	4-113	5- 1 ³ / ₄	5- 418	5- 6½	5-111	6-4	6- 83	7- 11/2	7- 61	7-11	8- 33	8- 8	9- 14	9-6	19
20	5-0	5- 21/2	5- 5	5- 7½	5-10	6- 3	6-8	7- 1	7- 6	7-11	8- 4	8- 9	9- 2	9- 7	10-0	20
21	5-3	5-5 ⁸	5-81/4	5-107	6- 11/2	6- 63	7-0	7- 51/4	7-101	8- 33	8- 9	9- 21/4	9- 71/2	10- O ₄	10-6	21
22	5-6	5-83	5-11½	6- 21/4	6- 5	6-101	7-4	7- 91	8- 3	8- 81	9- 2	9- 71/2	10- 1	10- 61	11-0	22
23	5-9	5-11 ⁷ / ₈	6- 23/4	6- 5 5	6-81/2	7- 21/4	7-8	8- 13/4	8- 71/2	9- 1-4	9- 7	10- O ₄	10- 61	11- O ₄	11-6	23
24	6-0	6- 3	6- 6	6- 9	7- 0	7- 6	8-0	8- 6	9-0	9-6	10- 0	10- 6	11-0	11- 6	12-0	24
25	6-3	6- 61/8	6- 91/4	7- 03	7- 31/2	7- 93	8-4	8-104	9- 41/2	9-104	10- 5	10-11	11- 51	11-113	12-6	25
26	6-6	6- 91/4	7- 01/2	7- 33	7- 7	8- 11/2	8-8	9- 21/2	9- 9	10- 31/2	10-10	11- 41	11-11	12- 51	13-0	26
27	6-9	7- 0 ³ / ₈	7- 33/4	7- 71/8	7-101	8- 51	9-0	9- 64	10- 11/2	10- 81	11- 3	11- 94	12- 41	$12-11\frac{1}{4}$	13-6	27
28	7-0	7- 31/2	7- 7	7-101/2	8- 2	8- 9	9-4	9-11	10- 6	11- 1	11-8	12- 3	12-10	13- 5	14-0	28
29	7-3	7- 6 ⁵	7-101/4	8- 17/8	8- 51/2	9- 03	9-8	10- 3 ¹ / ₄	10-10 ¹ / ₂	11- 53	12- 1	12- 84	13- 31/2	13-10 3	14-6	29
30	7-6	7- 93/4	8- 11/2	8- 51/4	8- 9	9- 41/2	10-0	10- 71/2	11- 3	11-101	12- 6	13- 11/2	13- 9	14- 41/2	15-0	30
SPACES	3	$3\frac{1}{8}$	$3\frac{1}{4}$	$3\frac{3}{8}$	31/2	$3\frac{3}{4}$	4	41/4	$4\frac{1}{2}$	434	5	54	$5\frac{1}{2}$	534	6	SPACES
SP,							PITC	H IN INC	HES							SP

EXTREME LENGTHS OF PLATES

(Rolled by Carnegie Steel Co.)

			U.	M.	PLA [*]	TES													SHE	EARE	D P	LA.T	ES							
	TH IN	01	14	91	08	32	30	35	40	45	48	26	64	33	92	80	84	06	96	100	105	801	110	112	115	118	130	WIDT		
	I_2^L	0-04	0-04	0-09	0-09	40-0	30-0	25-0	25-0	25-0	25-0	21 - 8	18 - 9	16 - 8	16 - 3	15 - 5	14 - 7	12 - 11	12 - 1	11-8	10 - 01	9-2		9 - 4	E			12		
	I_4^L	0-04	0-04	0 - 99	0-09	0-09	40-0	35-0	30-0	27-0	27-0	22 - 1	19 - 2	17 - 1	18 - 3	155	14 - 7	12 - 11	12 - 1	11-8	10 - 5	.10 - 0	9-2	9 - 4				I_{4}^{I}		
	I_8^I	0-04	0-04	0- 99	0-99	0-09	40-0	40-0	35 - 0	30-0	30-0	25 - 0	21-8	19 - 2	18 - 4	17 - 6	15 - 10	14 - 7	13 - 4	12 - 6	12 - 1	11-8	10 - 01	10 - 01	10 - 01	10 - 01	10 - 0	$I_{\overline{8}}^{I}$		
	1	0-04	75-0	0-08	0-94	65 - 0	20-0	45 - 0	40-0	35-0	35 - 0	27 - 6	24-2	20 - 10	20 - 02	19 - 2	17 - 11	16 - 3	15 - 0	14 - 2	13 - 4	12 - 6	10-10	10 - 5	10 - 5	10 - 0	10-0	1		dth.
	5/8	0-04	75-0	0-08	0-08	70-0	0-09	50-0	20-0	20-0	20-0	30 - 0	26 - 8	22 - 6	21 - 8	19 - 2	18 - 4	17 - 1	15 - 10	15 - 0	14 - 2	13 - 4	11-8	11-3	10 - 10	10 - 5	10 - 5	≥- ∞		reater wi
	13											31 - 8	28 - 4	25 - 0	23 - 4	21 - 8	20 - 5	17 - 11	16 - 8	15 - 10	15 - 0	14 - 2	12 - 6	12 - 6	11-8	10 - 10	10 - 5	13		he next g
CHES	8 4	0-04	75-0	80-0	0-08	75-0	65 - 0	65-0	65 - 0	0- 99	0- 29	34-2	30-0	26-8	25 - 0	23 - 4	20-10	18 - 4	17 - 6	16-8	15 - 10	15 - 0	13 - 4	12 - 11	12 - 6	10 - 10	10 - 5	24	CHES	ren use tl
THICKNESS IN INCHES	$\frac{9T}{11}$			Đ		٠						35 - 10	31-8	27- 6	25 - 10	23 - 4	21-8	20-0	18-4	16-8	15 - 10	15 - 0	13 - 4	12 - 11	12 - 6	11-3	10 - 10	111	THICKNESS IN INCHES	hs not gi
THICK	क्ष	0-04	85 - 0	85 - 0	0-28	85 - 0	85 - 0	80-0	0-08	0-08	0 - 08	40-0	33 - 4	29 - 2	27 - 6	25 - 10	25 - 0	21 - 8	20-02	18 - 4	16 - 8	15 - 0	13 - 4	13 - 4	11 - 8	11-3	10 - 10	20/00	THICKN	iate widt
	91	0-04	85 - 0	85 - 0	0-06	0-06	0-08	0-06	0-06	0 - 08	80 - 0	41 - 8	37 - 6	32 - 6	30 - 10	30 - 0	28-4	24 - 2	22 - 6	19 - 2	17 - 6	15 - 10	14-2	13 - 9	11 - 8			91		NOTE: For intermediate widths not given use the next greater width.
	H &	0-04	85 - 0	85 - 0	95 - 0	95-0	95-0	0-06	0-06	0 - 08	0 - 08	41 - 8	40 - 0	34 - 2	32 - 6	30 - 10	30-0	27 - 6	24 - 2	20 - 0	18 - 4	16-8	14 - 2	13 - 4				7 8		TE: For !
	7 16	0-04	75-0	0 - 08	100 - 0	100 - 0	0 - 06	90-0	0 - 08	80-0	0 - 08	41-8	40 - 0	35 - 10	35 - 0	33 - 4	31 - 8	28 - 4	25 - 10	20 - 5	19 - 2	16-8	13 - 4					16		ON
	8 80 - 0 1 100 - 0 1 100 - 0 1 100 - 0 1 100 - 0 1 100 - 0 100 10										70 - 0	41-8	38 - 4	35 - 0	33 - 4	31 - 8	30 - 0	27 - 6	25 - 10	16 - 8	16 - 8	14-2						∞ ∞		
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	TH IN	10	74	91	30	25.	30	35	07	45	48	26	79	3,0	92	80	84	90	96	100	105	801	011	112	115	118	120	WIDT		
			U	. M.	PLA	TES	5												SH	EAF	RED	PLA	TES							

DECIMAL PARTS.

7.	DECIMAL PARTS OF A FOOT DECIMAL PARTS OF													
	0	1	2	3	4	5	6	7	8	9	10	11	AN INCH	
	.0	.0833	.1667	.2500	.3333	.4167	.5000	.5833	.6667	.7500	.8333	.9167		
1 32	.0026	.0859	.1693	.2526	.3359	.4193	.5026	.5859	.6693	.7526	.8359	.9193	$\frac{1}{32}$.0313
1 16	.0052	.0885	.1719	.2552	.3385	.4219	.5052	.5885	.6719	.7552	.8385	.9219	1 16	.0625
$\frac{3}{32}$.0078	.0911	.1745	.2578	.3411	.4245	.5078	.5911	.6745	.7578	.8411	.9245	$\frac{3}{32}$.0938
1/8	.0104	.0938	.1771	.2604	.3438	.4271	.5104	.5938	.6771	.7604	.8438	.9271	1/8	.125
$\frac{5}{32}$.0130	.0964	.1797	.2630	.3464	.4297	.5130	.5964	.6797	.7630	.8464	.9297	$\frac{5}{32}$.1563
3 16	.0156	.0990	.1823	.2656	.3490	.4323	.5156	.5990	.6823	.7656	.8490	.9323	3	.1875
7 32	.0182	.1016	.1849	.2682	.3516	.4349	.5182	.6016	.6849	.7682	.8516	.9349	$\frac{7}{32}$,2188
1/4	.0208	.1042	.1875	.2708	.3542	.4375	.5208	.6042	.6875	.7708	.8542	.9375	14	.25
$\frac{9}{32}$.0234	.1068	.1901	.2734	.3568	.4401	.5234	.6068	.6901	.7734	.8568	.9401	$\frac{9}{32}$.2813
$\frac{5}{16}$.0260	.1094	.1927	.2760	.3594	.4427	.5260	.6094	.6927	.7760	.8594	.9427	5 16	.3125
11 32	.0286	.1120	.1953	.2786	.3620	.4453	.5286	.6120	.6953	.7786	.8620	.9453	$\frac{11}{32}$.3438
3/8	.0313	.1146	.1979	.2813	.3646	.4479	.5313	.6146	.6979	.7813	.8646	.9479	3 8	.375
$\frac{13}{32}$.0339	.1172	.2005	.2839	.3672	.4505	.5339	.6172	.7005	.7839	.8672	.9505	$\frac{13}{32}$.4063
7 16	.0365	.1198	.2031	.2865	.3698	.4531	.5365	.6198	.7031	.7865	.8698	.9531	7 16	.4375
$\frac{15}{32}$.0391	.1224	.2057	.2891	.3724	.4557	.5391	.6224	.7057	.7891	.8724	.9557	$\frac{15}{32}$.4688
1/2	.0417	.1250	.2083	.2917	.3750	.4583	.5417	.6250	.7083	.7917	.8750	.9583	1/2	.5
$\frac{17}{32}$.0443	.1276	.2109	.2943	.3776	.4609	.5443	.6276	.7109	.7943	.8776	.9609	17 32	.5313
9 16	.0469	.1302	.2135	.2969	.3802	.4635	.5469	.6302	.7135	.7969	.8802	.9635	9 16	.5625
$\frac{19}{32}$.0495	.1328	.2161	.2995	.3828	.4661	.5495	.6328	.7161	.7995	.8828	.9661	19 32	.5938
58	.0521	.1354	.2188	.3021	.3854	.4688	.5521	.6354	.7188	.8021	.8854	.9688	<u>5</u>	.625
$\frac{21}{32}$.0547	.1380	.2214	.3047	.3880	.4714	.5547	.6380	.7214	.8047	.8880	.9714	$\frac{21}{32}$.6563
11 16	.0573	.1406	.2240	.3073	.3906	.4740	.5573	.6406	.7240	.8073	.8906	.9740	11 16	.6875
23 32	.0599	.1432	.2266	.3099	.3932	.4766	.5599	.6432	.7266	.8099	.8932	.9766	$\frac{23}{32}$.7188
3	.0625	.1458	.2292	.3125	.3958	.4792	.5625	.6458	.7292	.8125	.8958	.9792	34	.75
25 32	.0651	.1484	.2318	.3151	.3984	.4818	.5651	.6484	.7318	.8151	.8984	.9818	$\frac{25}{32}$.7813
13 16	.0677	.1510	.2344	.3177	.4010	.4844	.5677	.6510	.7344	.8177	.9010	.9844	13 16	.8125
27 32	.0703	.1536	2370	.3203	.4036	.4870	.5703	.6536	.7370	.8203	.9036	.9870	$\frac{27}{32}$.8438
7 8	.0729	.1563	.2396	.3229	.4063	.4896	.5729	.6563	.7396	.8229	.9063	.9896	7	.875
$\frac{29}{32}$.0755	.1589	.2422	.3255	.4089	.4922	.5755	.6589	.7422	.8255	.9089	.9922	32	.9063
16	.0781	.1615	.2448	.3281	.4115	.4948	.5781	.6615	.7448	.8281	.9115	.9948	15 16	.9375
$\frac{31}{32}$.0807	.1641	.2474	.3307	.4141	.4974	.5807	.6641	.7474	.8307	.9141	.9974	$\frac{31}{32}$.9688

3 Spans, Single Trock, Through 175:0"c.c. End Pins. Bridge # 720 Big Run, Evansville, Arizona. M. K. and T. RY. MIDDLE DIV.

TOP CHORDS AND END POSTS.

Scole 34"=1A.

AMERICAN BRIDGE CO.,

EDGEMOOR PLANT.

A. B. Co. Contr. No. 932 In Charge of Wilson.

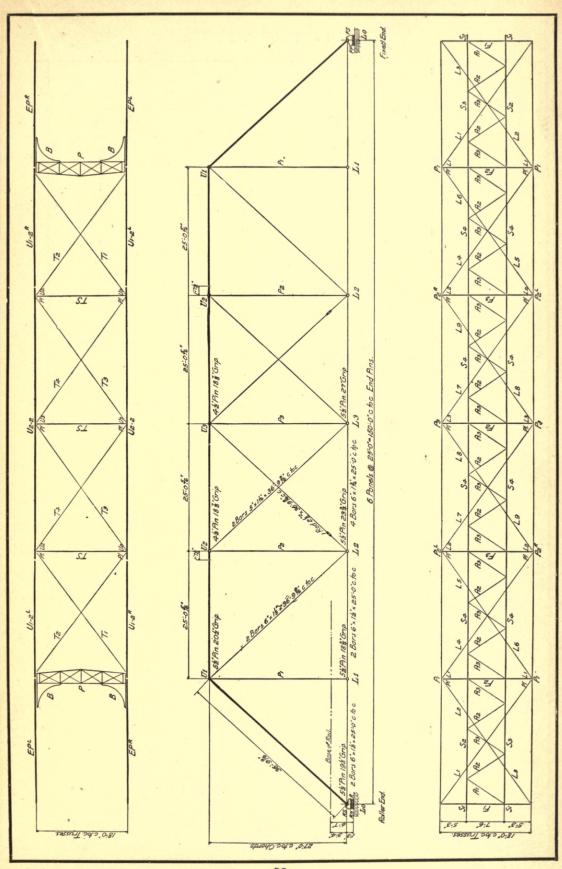
Date 9/25/00 Rev. 9/30 Made by J. C.

Checked by E.K. Date 9/27/00 Rev.

ORDER NO. K230 "SHEET NO. 12

Cut Blue Print onthis line

Cut Tracing on this line



ATLANTIC MUTUAL INS. CO. BLDG. NEW YORK. 2 MP. FLOOR GIRDERS #3, 4, 5 AND 6.

Scale 3/4 = 1 Foot.

AMERICAN BRIDGE CO.,

EDGEMOOR PLANT.

A. B. Co. Contr. No._____In Charge of._

Made by Date....

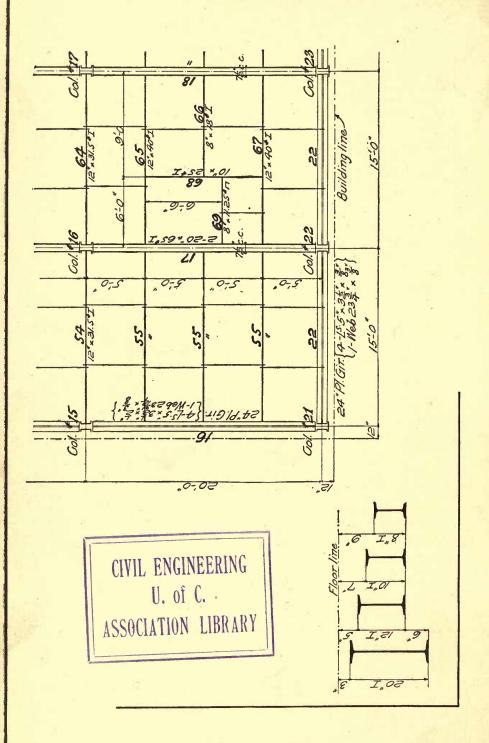
Checked by.....Date....

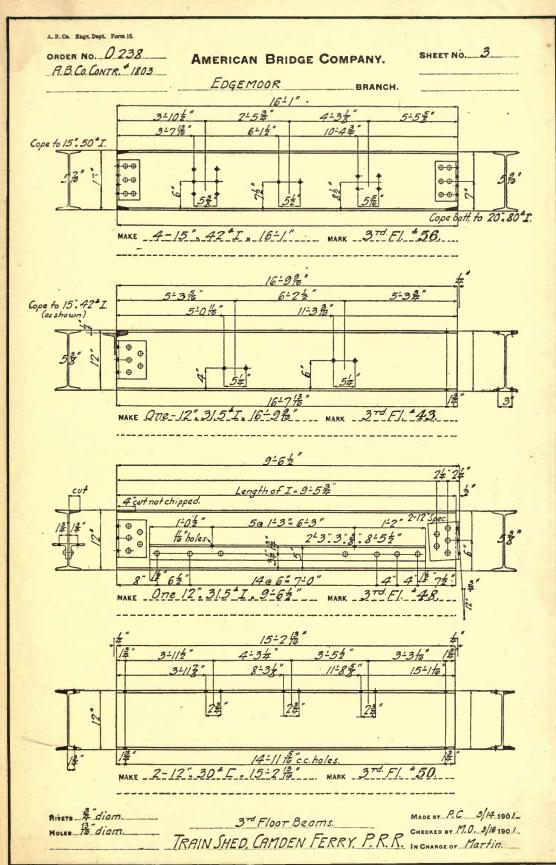
SHEET NO.

ORDER NO.

	Roof	1	2	3	4	5	
Variable	17 th Floor	08.8	8.8	8:11	116) 811	115) 8.8	
14:0"	16 th Floor	2-10"x15"E 24"x 76" LOCING	90.	2-10"x20#L 24"x F6" LOCING	90.	Some os #1	
14:0"	15th Floor	\$ 28-0-Fin. 15.1°	82 151	} 28-0-Fin.	83)	15.1	
.0.51	14 th Floor	2-10"x 15." [2-10" x 70 COV. pls	90.	2-10"x 20:# [2-10"x 3" cov.pls.	do.	some os #1	
NOTE:	Figures in	J de	enote si	heet nu	mbers		

LAY-OUT FOR BUILDING-WORK.



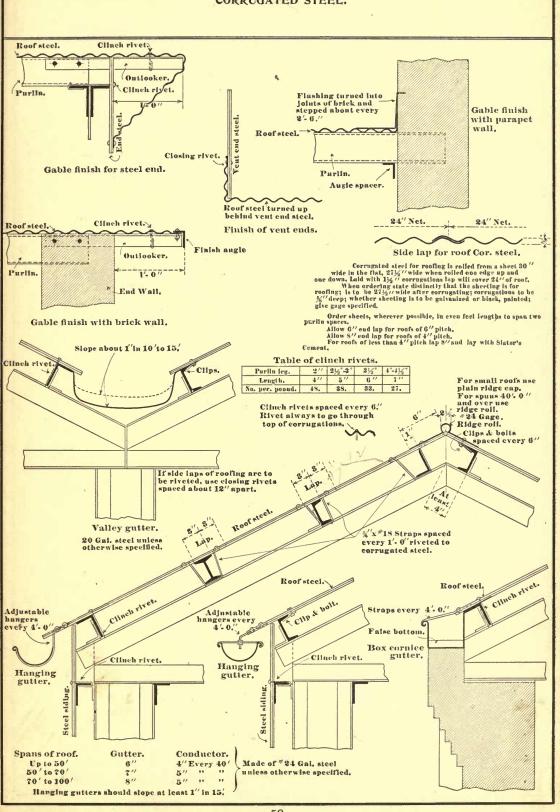


	1608	LASSIG.	BRANCH.	
	3 ¹ 5°	5 ' 2\$"	4:35"	10
T	3-5 3-54	6-12"	3-6"	16.
	2-1"	4-23	6+22"	17.
_		4-14	0-22	
12-U#"		9-35	4-03"	6.
13-04		9-43	3-33	
12-64"	3"	10-25"	5-113	3 set back
	1		1-6:6:76:0-5	-
	→		⊕ ⊕	
9	+	+	⊕ ⊕	11 11
		54 5.	#]	
M.	AKE OTTE 10:15 [. 1	12-114 (ord. 12-102) MA	RX 4th El. # 6	
m	"	3-03"(" 13-0")	4th Fl 17	
•••	"	2-64" (" 12-52) "	4th Fl 27	
		4.94.514.226		
16				
				"
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47				
470	= =	4400		
. 450.	5-0"	4:3"	5 <u>+3</u> "	-
132	4:9"	4-52"	6 <u>+</u> 2"	
132	4:9"	4-52"	6 <u>+</u> 2"	
132 143	4-9" 4-5"	4:5½° 5:0"	6 <u>+</u> 2"	1016
*32 *43	4-9" 4-5" 4-93"	4:5½. 5:0" 9:0¾"	6 <u>+</u> 2"	14-6
'32 '43 '50 '32	4-9" 4-5" 4-9\frac{2}{4}" 4-6\frac{2}{4}"	4:5½. 5:0" 9:0¾" 9:0¼"	6 <u>+</u> 2"	15-4
*32 *43	4-9" 4-5" 4-93"	4:5½. 5:0" 9:0¾"	6 <u>+</u> 2"	
'32 '43 '50 '32 '43	4:9" 4:5" 4:0\frac{2}{4}" 4:2\frac{2}{4}"	4:5½. 5:0" 9:0¾" 9:0¼" 9:2¾"	6 <u>+</u> 2"	15-4
'32 '43 '50 '32 '43	4-9" 4-5" 4-9\frac{2}{4}" 4-6\frac{2}{4}"	4:5½. 5:0" 9:0¾" 9:0¼" 9:2¾"	6 <u>+</u> 2"	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:6\$\frac{3}{4}" 4:6\$\frac{3}{4}" 4:2\$\frac{3}{4}"	4:5½. 5:0" 9:0¾" 9:0¼" 9:2¾"	6 <u>+</u> 2"	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:6\$\frac{3}{4}" 4:6\$\frac{3}{4}" 4:2\$\frac{3}{4}"	4:5½. 5:0" 9:0¾" 9:0¼" 9:2¾"	6 <u>+</u> 2"	15-4
'32 '43 '50 '32 '43	4-9 ² " 4-9 ² " 4-6 ² " 4-2 ² " 4-2 ² " 2 ¹ 6-6-7-0-3" 2 ¹ 8-1-2 ² "	4 - 5 = 0 $5 - 0$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 0 $5 - 0$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
'32 '43 '50 '32 '43	4-9 ² " 4-9 ² " 4-6 ² " 4-2 ² " 4-2 ² " 2 ¹ 6-6-7-0-3" 2 ¹ 8-1-2 ² "	4 - 5 = 2 $5 - 0$ $9 - 0 = 2$ $9 - 0 =$	6 <u>+</u> 2"	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 0 $5 - 0$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
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'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 2 $5 - 0$ $9 - 0 = 2$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
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'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 2 $5 - 0$ $9 - 0 = 2$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 2 $5 - 0$ $9 - 0 = 2$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4
'32 '43 '50 '32 '43	4:9" 4:5" 4:9\$" 4:6\$" 4:2\$" 4:2\$" 2bolts \$\frac{3}{4}:0.2\$" AKE One 12:40 4	4 - 5 = 2 $5 - 0$ $9 - 0 = 2$ $9 - 0 =$	6 ¹ 2" 5 ¹ /(³ / ₄ "	15-4

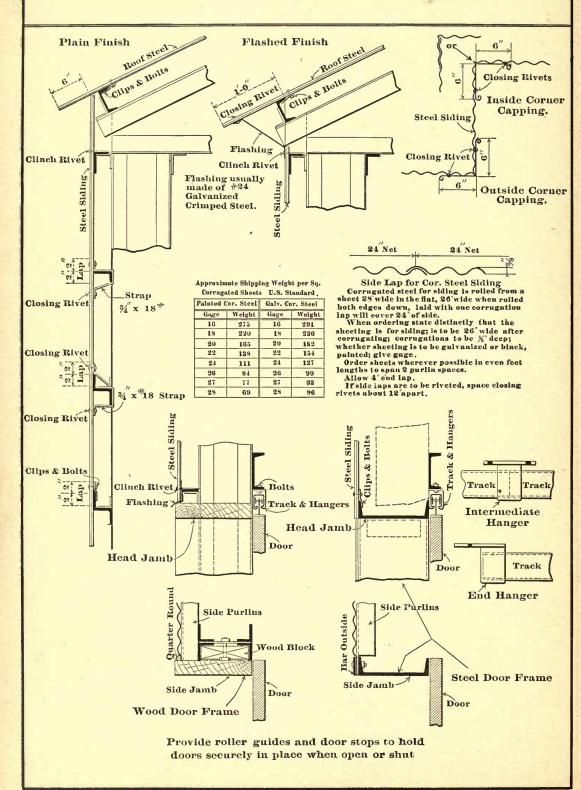
A.B. Oa. Engr Dept. Fo	1592	AMERICAN	BRIDGE C	OMPANY.	SHEET NO	32
A.B.Co. CONTR	1806	SHIFFLER. BRANCH.			**************************************	45 FF (+) 21 CO + 10 C
GARAGO SAN MINISTER PROPERTY FROM CONTINUES		JAII	FLEN.	BRANCH.	enteres en como i i disconsidentes admi	
4	•	•	17-0		#	
-	1-0"	5-0	5÷0°	5÷0"	1-0"	
		6-0	11-0	16-0		4
(5,"	. Jep.	Sep	- Jap	Seo		
12 62cc 13	4	/	6-82 c.c. hole.	S.	/孝 "	
. 10	0	ne Girder Mo	rk 2nd F/.	<i>*6</i>		
		2-15" 50 4 4-C/ Seps 1				
		8- # Bolts.	0-8 19			
			16-3"			
	· /÷0"	4-9"	4:9"	4:9"	1-0-	
		5-9"	10-6"	15-3"		
TTI						TT
5	Sep	des	Sep			
	annag -	*			· · · · · · · · · · · · · · · · · · ·	
in L		One Girder	Mark 2nd	F/ 29.		
		2-15-50 4-01 Sea	= 15 N-6"10	Ship Loose.		
		One Girder 2-15 50 4- C.I. Sep. 8- \$ Bols	15 0'8 lg.			
						
						TT
					7	
		6				
9/7 H	THE PARKET WITH SPECIFICATION	2nd. F	loor Beams.		MADE BY A.B.	
House To dian	7	NION PASSEN	GER STA P	TITSBURGH	CHECKED BY X.Y.	9/27.1900.
TRINT. Une co	hite.	INIOIN I MUSE IN	1413 21115	III DUNGILL	IN CHARGE OF	manuscript 110

A. B. Co. Engr Dept. Form 21 ORDER NO. 2720 SHEET NO. 18 AMERICAN BRIDGE COMPANY. A.B.Co. CONT \$ 1837. BROOKLYN BRANCH. 0000 0 0 0 ⊕ ⊕ 126 MAKE 2-20-80 Is 18-63 Ord 18-58 MAKE 2-12"31 0=1年 1-3" 3-75 18-6 53 14:38 14-07 18-8 4-5 MADE BY Fy F 8/28 190 /CHECKED BY E.K. 8/30 190 /-3 diam. First Floor Beams. HoLES 76 diamy unlass MOTKED OTHERWISE. U. S. CUSTOM HOUSE. NEW YORK. IN CHARGE OF KTUEGET.

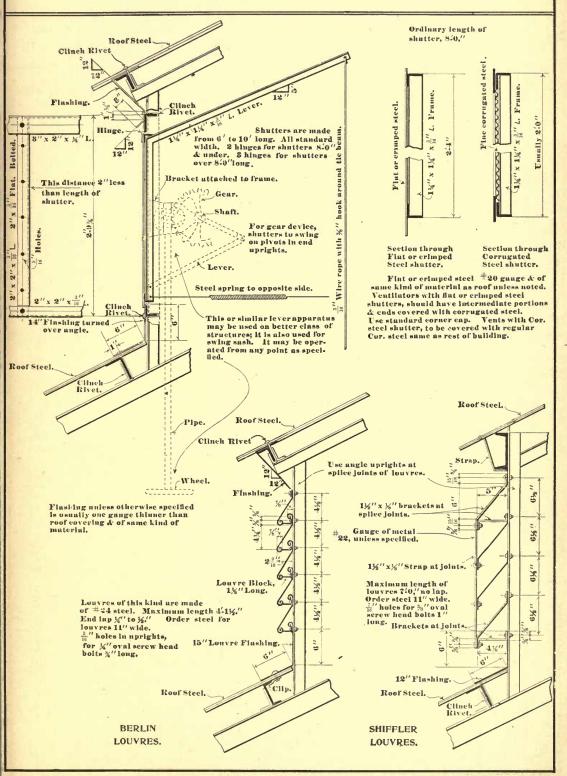
CORRUGATED STEEL.



CORRUGATED STEEL



CORRUGATED STEEL

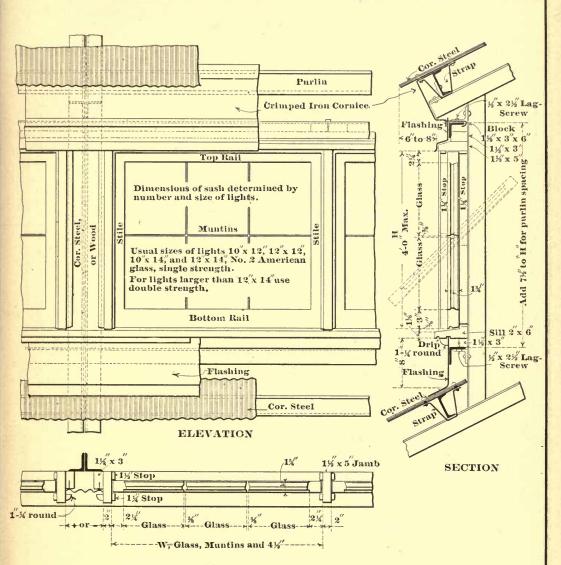


ORDINARY WINDOW SASH

Dimensions in feet and inches,

				Dimensions in	leet and inches.				
NO OF LIGHTS	SIZE OF GLA9S	WIDTH	HEIGHT	STYLE	STYLE	HEIGHT	WIDTH	SIZE OF	NO. OF LIGHTS .
6	10 x 12 12 x 12 10 x 14 12 x 14 10 x 16 12 x 16 14 x 16	2-11\frac{1}{4} 3-5\frac{1}{4} 2-11\frac{1}{4} 3-5\frac{1}{4} 3-5\frac{1}{4} 3-5\frac{1}{4} 3-5\frac{1}{4} 3-11\frac{1}{4}	2 - 5 \(\frac{5}{8} \) 2 - 5 \(\frac{5}{8} \) 2 - 5 \(\frac{5}{8} \) 2 - 9 \(\frac{5}{8} \) 3 - 1 \(\frac{5}{8} \)			$4 - 7\frac{1}{2}$ $4 - 7\frac{1}{2}$ $5 - 3\frac{1}{2}$ $5 - 3\frac{1}{2}$ $5 - 11\frac{1}{2}$ $5 - 11\frac{1}{2}$ $5 - 11\frac{1}{2}$	2 - 11\frac{1}{3} 3 - 5\frac{1}{4} 2 - 11\frac{1}{4} 3 - 5\frac{1}{4} 2 - 11\frac{1}{4} 3 - 5\frac{1}{4} 3 - 11\frac{1}{4}	10×12 12×12 10×14 12×14 10×16 12×16 14×16	12
9	10 × 12 12 × 12 10 × 14 12 × 14 10 × 16 12 × 16 14 × 16	2-11 ¹ / ₄ 3-5 ¹ / ₈ 2-11 ¹ / ₄ 3-5 ¹ / ₄ 3-5 ¹ / ₂ 3-11 ¹ / ₄	3-6 3-6 4-0 4-0 *4-6 *4-6			6 - 8 \frac{1}{4} 6 - 8 \frac{1}{4} 7 - 8 \frac{1}{4} 7 - 8 \frac{1}{4} 8 - 8 \frac{1}{4} *8 - 8 \frac{1}{4} *8 - 8 \frac{1}{4}	2 - 11\frac{1}{4} 3 - 5\frac{1}{4} 2 - 11\frac{1}{4} 3 - 5\frac{1}{4} 2 - 11\frac{1}{4} 3 - 5\frac{1}{4} 3 - 11\frac{1}{4}	10 × 12 12 × 12 10 × 14. 12 × 14 10 × 16 12 × 16 14 × 16	18
8	10 × 12 12 × 12 10 × 14 12 × 14 •10 × 16 12 × 16 14 × 16	3 - 9 \(\frac{5}{6} \) *4 - 5 \(\frac{5}{6} \) 3 - 9 \(\frac{5}{6} \) *4 - 5 \(\frac{5}{6} \) *4 - 5 \(\frac{5}{6} \) *4 - 5 \(\frac{5}{6} \) *5 - 1 \(\frac{5}{6} \)	2 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 - 5 -			$4 - 7\frac{1}{2}$ $4 - 7\frac{1}{2}$ $5 - 3\frac{1}{2}$ $5 - 3\frac{1}{2}$ $5 - 11\frac{1}{2}$ $5 - 11\frac{1}{2}$ $5 - 11\frac{1}{2}$	3 - 9 5 5 6 3 - 9 5 6 8 4 - 5 5 6 3 - 9 5 7 8 8 8 8 8 5 - 1 5 6 6 8 5 7 1 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	10×12 12×12 10×14 12×14 10×16 12×16 14×16	16
12	10×14 12×14 10×16		3-6 3-6 4-0 4-0 *4-6 *4-6	The state of the s		6 - 8 \frac{1}{4} 6 - 8 \frac{1}{4} 7 - 8 \frac{1}{4} 7 - 8 \frac{1}{4} 8 - 8 \frac{1}{4} 8 - 8 \frac{1}{4} 8 - 8 \frac{1}{4}	3 - 9 \frac{5}{6} \\ * 4 - 5 \frac{5}{6} \\ 3 - 9 \frac{5}{6} \\ * 4 - 5 \frac{5}{6} \\ * 5 - 1 \frac{5}{6} \\ * 5	10 × 12 12 × 12 10 × 14 12 × 14 10 × 16 12 × 16 14 × 16	24

CONTINUOUS FRAMES AND SASH IN MONITOR.

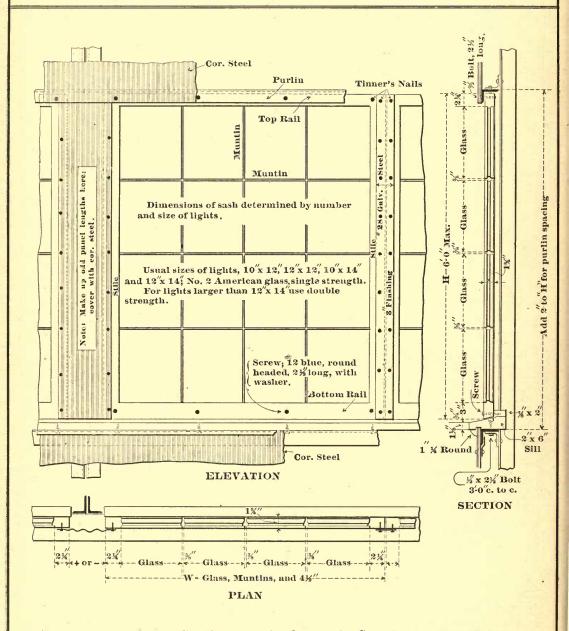


PLAN

Design shown is for fixed sash for monitor; for swing monitor sash, cut stops off as shown by dotted lines and omit head stop on inside.

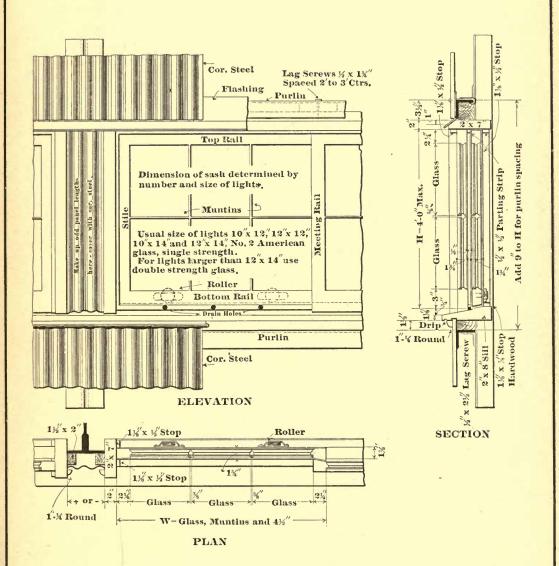
Make frames and sash of White Pine, excepting spiking and blocking pieces, which are to be of Spruce, Hemlock or Norway Pine, planed on all exposed sides. For swing sash order two trunnions for each sash, and call for lever operating device.

CONTINUOUS SASH



Design shown is for Continuous Fixed Sash in Cor. Steel sides. Make sash and sill of white Pine, planed on all exposed sides.

CONTINUOUS SLIDING SASH.

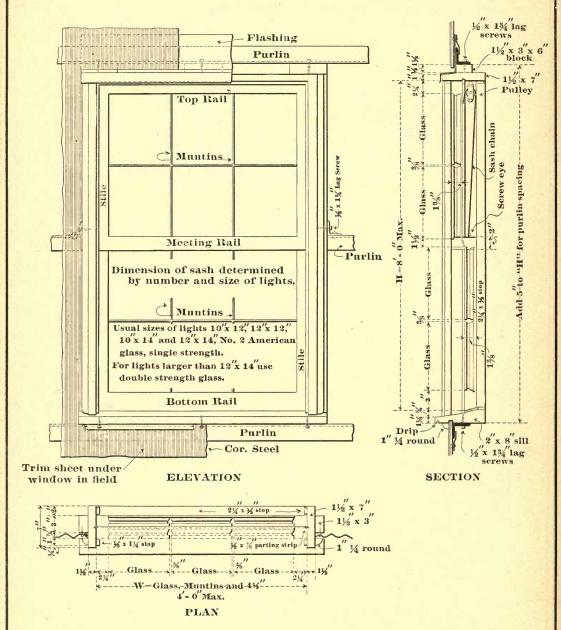


Design shown is for sliding sash in corrugated steel sides.

Make frames and sash of white pine, excepting spiking and blocking pieces, which are to be of spruce, hemlock or Norway pine, planed on all exposed sides. The stop used for roller track, is to be hardwood.

Call for operating device, as desired.

COUNTERBALANCED WINDOWS

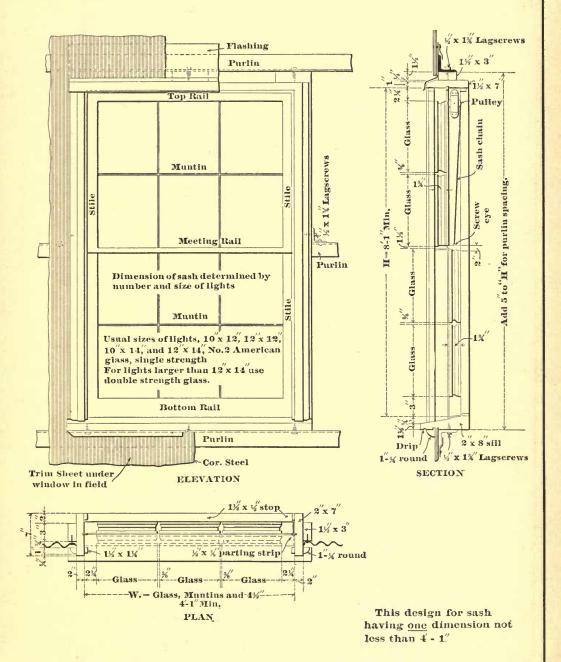


Design shown is for a window frame with counterbalanced sash in corrugated steel sides,

Make frame and sash of white Pine, except spiking and blocking pieces, which are of Spruce, Hemlock or Norway Pine planed on all exposed sides.

This design for sash having both dimensions not greater than 4'-0".

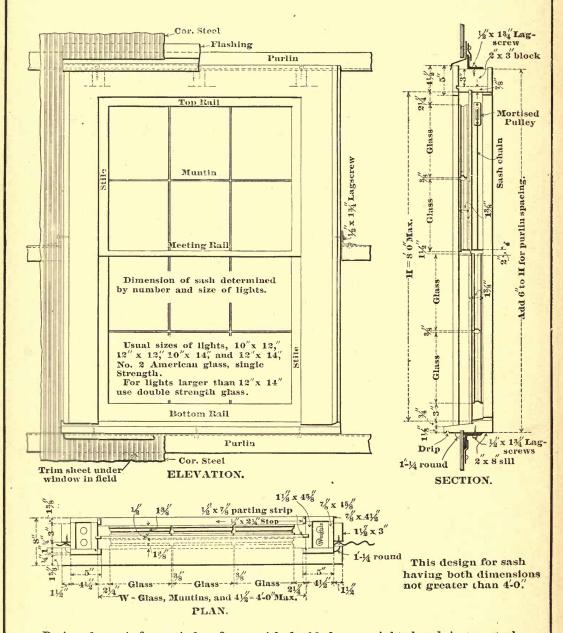
COUNTERBALANCED WINDOWS



Design shown is for a window frame with counterbalanced sash in corrugated steel sides.

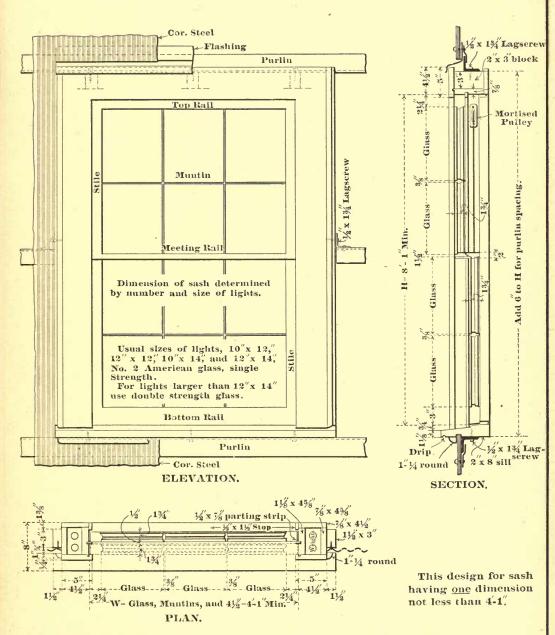
Make frame and sash of white Pine, except spiking and blocking pieces, which are of Spruce, Hemlock or Norway Pine, planed on all exposed sides.

DOUBLE HUNG, WEIGHTED WINDOWS.



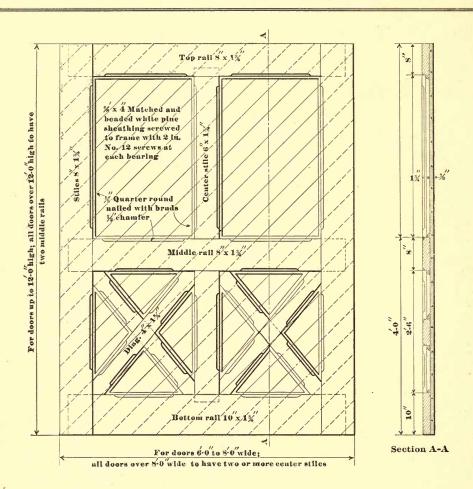
Design shown is for a window frame with double hung weighted sash in cor. steel siding. Make sill and casings of white pine; jambs and parting strips of hard pine; spiking pieces and blocks of Spruce, Hemlock or Norway Pine, planed on all exposed sides.

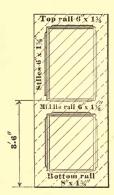
DOUBLE HUNG, WEIGHTED WINDOWS.



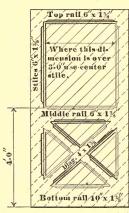
Design shown is for a window frame with double hung weighted sash in cor. steel siding. Make sill and easings of white pine; jambs and parting strips of hard pine; spiking pieces and blocks of Spruce, Hemlock or Norway Pine, planed on all exposed sides.

STANDARD DOOR

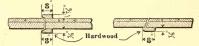




Design for door up to 3'-0"x 7'-0"



Design for doors over 3'-0"x 7'-0" and up to 6'-0"wide

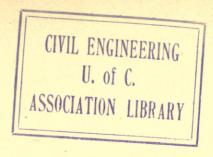


Meeting strips for double sliding doors. Meeting strip for double swing doors.

Doors may be either slide or swing: Sliding doors should be 4 wider and 2 higher than clear opening between Jambs, All doors under 6'0' wide to have 1% stilles and rails. All doors over 6'0' wide to have 1% stilles and rails. All stilles und rails to be halved or mortised and tenoned together.

Doors to be made of white pine

If doors are to be covered with the or sheet metal they are to be made of two or more thicknesses of % matched white pine sheathing not over 4 wide, laid diagonally and put together with wrought nalls well clinched.



CORRUGATED SHEETING FOR BUILDINGS AND ROOFS.

Two kinds of corrugated sheeting are used in covering buildings—the flat, which is to be painted, and the galvanized. In the United States sheets both flat and corrugated are made by the mills to fulfil the standards, fixing the gage, thickness and weight per square foot, approved by act of Congress, March 3, 1893. They are as follows:

		W	EIGHT PER	SQUARE FOO	T.
GAUGE No.	THICKNESS IN INCHES.	FLAT	SHEETS.	Corrugatei	SHEETS.
		Black.	Galvanized.	Black, Painted	Galvanized.
16	.0625	2.50	2.66	2.75	2.91
18	.0500	2.00	2.16	2.20	2.36
20	.0375	1.50	1.66	1.65	1.82
22	.0313	1.25	1.41	1.38	1.54
24	.0250	1.00	1.16	1.11	1.27
26	.0188	0.75	.91	0.84	0.99
28	.0156	0.63	.79	0.69	0.86

The weights of corrugated in the above table are for the standard corrugations, $2\frac{1}{2}$ inches wide approximately, and $\frac{5}{8}$ of an inch deep.

The standard slope for roofs covered with corrugated sheeting is six inches to the foot. The sheeting should not be used on roofs having a slope of less than four inches to the foot, unless special provision be made to make laps tight. The sheeting is placed directly on

Roofing.

purlins spaced proper distances apart, or may be laid directly on wood sheathing, which covers the entire surface of the roof. When the corrugated sheeting rests on purlins, the gages commonly used are No. 20 and 22.

Siding.

Corrugated sheeting for siding of buildings is attached the same as roofing, directly to the purlins or nailed to wood sheathing. If purlins are used, the gages commonly specified are Nos. 22 and 24. One gage lighter being used for the siding than is used for the roofing for the same building.

Whenever possible, particularly for roofing, sheets should be arranged to span at least two purlin spaces.

Fastening for Corrugated Sheeting.

Various methods of fastening the corrugated sheeting to purlins on the sides and roofs of buildings are shown on the accompanying standard illustrations, pages No. 58 and 59, and in a general way is shown the different conditions under which the different methods of fastening should be employed.

Straps

Straps made of band steel ³ inch wide are used to fasten the sheeting to purlins made of all the rolled shapes, but usually are to be applied for fastening the sheeting on roofs where channel, I Beam or Z-bar purlins are used. These straps pass around the purlins, and are riveted at both ends to the sheets, or they may be fastened by bolts specially made for such purpose. Two of these straps should be used for each sheet on each purlin, or practically 12 inches apart.

Clinch Rivets.

Clinch rivets are commonly used for attaching corrugated sheeting to angle iron purlins. These rivets are made of wire with a special head which fits the top of the

corrugation and, as indicated on pages Nos. 58, are put through the sheets close to the upper face of the angle purlin and clinched around its lower edge. The same fastening is used for siding where angle purlins or girts are employed. Three or four of these clinch rivets should be used for each sheet on each purlin or girt, spacing them practically 6 inches apart for roofing and 8 inches for siding.

Clips and bolts are used for fastening corrugated clips and Bolts. sheeting to channel, I Beam or Z-bar purlins, where straps or clinch rivets cannot be conveniently employed, or when steel sheeting is lined with an asbestos anti-condensation lining. The clips are made of flat steel, 1½ inches wide, about 2½ inches long, and are slightly crimped at one end to go over the flange of the purlin.

One bolt is used on each clip, and this bolt is made of the same diameter of wire, and has the same head made to fit the top of the corrugations as the clinch rivets. These clips and bolts are spaced 6 inches and 8 inches apart, the same as the clinch rivets.

Edges of sheets where side laps are made are usually riveted every 12 inches.

All fastenings should be securely applied, and the sheeting brought to snug bearing on purlins and at all joints.

In all cases the bolts or rivets fastening the sheeting to the purlins or girts should go through the tops of the corrugations.

Flashing, Ridging, Capping and Cornices.

Flashing, ridging, capping and cornices should always be used to cover up the joints in sheeting, and make the structure weatherproof. Flashing, when used

where the slope in the sheeting changes direction, should be of sufficient dimension and so arranged that at least three inches vertical height is obtained between the edge of the flashing and the end of the corrugated sheeting. Vertical seams of all flashing should be closely riveted, and the horizontal edges of the flashing should be securely riveted to the corrugated roof or side sheeting.

Ridging

Ridging should be placed covering the apex of all roofs, and where buildings are of ordinary size standard ridge roll should be used.

Corner Capping.

Corner capping is either bent, plain edge flat steel, or bent flat steel, with the edges terminating in a small scroll to keep the capping well in line, or corrugated sheeting may be turned around the corners neatly, thus closing up the opening where the two vertical surfaces join.

Cornices

Cornices along the eaves and the gable ends of buildings may be finished in various ways. The two usual methods employed for both eaves and gable ends are shown on pages No. 58, 59 and 60. If desired, a molded cornice can be used, made of such a form to fit the projection of the purlins, and of dimensions commensurate with the size and character of the building.

Gutters and Conductors.

Gutters and conductors are made of various dimensions, styles and forms. Three eave gutters in common use are sketched on page No. 58. Conductors are made of plain sheets with round cross-section, and corrugated of either round or rectangular cross-section. The round conductors are more commonly used.

Ventilators.

Openings in ventilators may be fitted with shutters, sash, or with louvers.

Shutters

Shutters are made of angle iron frames covered either with flat, crimped or corrugated sheets. These shutters are hinged at the top, and may be operated by a straight lever device, by means of cord and spring, or operated by means of any of the worm gear toggle arrangements which are commonly used in various localities for such purposes. Any device which will easily open and close the shutter, and at the same time securely hold it in any position in which it may be placed, will fulfil the required conditions.

Louvers

Louvers are usually made in two different ways. The louver shown on page No. 60 as the Shiffler Louver is one which is commonly used in the Pittsburg District, while the other form, known as the Berlin Louver, has been used in the East.

Tubular Ventilators.

Tubular ventilators of various kinds are often employed in place of monitors for roof ventilation. These tubular ventilators are made of various dimensions, of galvanized or other sheet metal, and are usually placed along the ridge line of the roof. For proper ventilation of the building, it is customary to estimate that one square foot of ventilator area will ventilate 300 to 400 square feet of floor area, depending on the character of building to be ventilated.

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Skylights.

When skylights are placed in roofs of structures, two types are used:

Box Skylights.

Box skylights covering a small area are placed in the slope of the roof at proper intervals. These are placed on a curb raising the glass above the roof line.

Continuous Skylights. Continuous skylights are made in an extended strip of a width sufficient to properly light the building interior. These are placed in the slope of the roof, preferably at the ridge.

Ribbed glass is used extensively for skylight work in thicknesses varying from 4 inch to 3 inch. The glass is supported by steel bars, either solid special rolled sections, or made of sheet metal properly formed to receive the glass. The supporting bars are spaced about 20 inches apart, to suit the sheets of the glass, which comes in sheets about 20 inches wide, and not more than 8 feet long.

Doors and Windows.

Doors.

Steel doors for corrugated steel buildings are made by covering an angle iron frame with corrugated sheeting, usually the same quality as the building siding.

Fireproof doors are sometimes constructed of two or more thicknesses of matched pine sheathing, covered on both sides with flat sheet steel or tin.

Wood doors are usually constructed of matched pine sheathing fastened to a well built frame, as shown in the details on page No. 69. Stock wood doors up to 3 feet wide can be procured of manufacturers, and are usually made of white pine with molded panels. Small

single doors up to 4 feet wide should be detailed to swing on hinges, and large doors, both single and double, should be arranged to slide sidewise on overhead trolley tracks with adjustable hangers, or to lift upwards between vertical guides, counterbalanced by weights attached to ropes passing over sheaves.

Doors should be detailed to suit the conditions they are to fill, and the openings they close.

The different types of windows ordinarily used in the sides of buildings, constructed with corrugated sheet siding, are shown on pages No.61 to 68. The sash and frames are constructed of white pine, and the glazing is usually No. 2 or A quality, American glass, single or double strength, depending on the size of the lights. The sizes of glass commonly used are 10 inches by 12 inches, 12 inches by 12 inches, 10 inches by 14 inches and 12 inches by 14 inches lights.

In the sides of buildings where light is to be obtained and no ventilation desired the continuous fixed sash is used. See page No. 63.

If a maximum amount of light is desired and ventilation is to be obtained, the continuous sliding sash can be used. See page No. 64. This detail allows one-half of the window area to be opened.

In the sides of monitors and sometimes in the sides of buildings, swing sash are used. See page No. 62. These should be carefully made and fitted, and operated by a device that will hold them securely in any position.

Two kinds of single windows with two sash each are shown by the sketches.

One is the Counterbalanced Window, where one sash balances the other. See page No. 65. 66. Two sizes are shown varying by thickness of sash for two sizes of windows.

Windows.

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CEPARTMENT OF CIVIL ENGINEER NO

Another is the Double-Hung Weighted Window, where each sash is balanced by weights, also arranged for two sizes of openings. See page No. 67 to 68.

The styles of windows shown on illustrations. will fulfil all the requirements desired for ordinary factory or mill building construction. For windows in brick walls the frames need only to be modified slightly to suit the usual details for brick work.

Corrugated Steel Arches.

Curved or arched corrugated sheets are used for arches between rolled beams, forming a support for concrete filling. The steel is ordinarily the standard 2½ inch corrugation, and gages are Nos. 16, 18, and 20, depending on the superimposed load and the length of span. The rise of the arch should not be less in inches than the span in feet, and should be determined by the depth of beam supports and the thickness of material allowed over crown of the arch, varying from 2 to 4 inches. Beams are spaced for this construction from 4 to 7 feet apart.

RULES

...for...

MAKING SHOP DRAWINGS.

The standard size of sheet shall be 24 by 36 inches, Size of Sheet. with two border lines ½ and 1 inch from the edge respectively. See page 49.

Small sheets shall be used for beams, pins, eye-bars, etc. Special forms are provided for these sheets.

The title shall be arranged uniformly for each contract near the lower right-hand corner of the sheet.

See pages Nos. 49 and 51.

A stamp is provided for the contract, sheet number, etc. It shall be applied in the lower right-hand corner of the sheet. The name of the draughtsman in charge of the work shall appear in full, others with initials only. See page No. 49.

Detail drawings shall as a rule be made in scale ³/₄ Scale. or 1 inch to the foot; for large plate and lattice girders ¹/₂ and ³/₅ inch may be used. Larger scales, such as 1¹/₂ and 3 inches to the foot, are permissible only for showing certain complicated details or for machine work.

Large sheets shall be neatly and carefully made to General Rules, exact scale.

Members shall be detailed in the position which they occupy in the structure, *i. e.*, horizontal members shall

be shown lengthwise, and vertical members crosswise on the sheet. Inclined members (and vertical ones when necessary on account of space) may be shown lengthwise on the sheet, but then always with their lower end to the left.

Avoid notes as much as possible. Where there is the least chance for ambiguity make another view.

Show all elevations, sections and views in their proper position—looking *toward* the member. Place the top view directly above and bottom view below the elevation. • The bottom view shall always consist of a horizontal section seen from above.

In sectional views the web or gusset plates shall always be blackened. Angles, fillers, etc., shall be cross-hatched, but only when necessary on account of clearness. In a plate girder for instance, it is not necessary to cross-hatch *all* the stiffeners and fillers in the bottom view.

Holes for field connections shall always be blackened, and shall, as a rule, be shown in all elevations and sectional views. Rivet heads shall be shown only when necessary; f. i., at the ends of members, around field connections, when countersunk, flattened, etc., etc.

In detailing members which adjoin or connect to others in the structure, part of the latter shall be shown in red, sufficiently to indicate the clearance required or the nature of the connection. Plain building work is exempt from this rule.

When part of one member is detailed same as another, figures for rivet spacing, etc., shall not be repeated; refer to previous sheet or sheets, bearing in mind that these must contain final information. It is not permissible to refer to a sheet, which in turn refers to

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another. Main dimensions, which are necessary for checking, such as c. to c. distances, story heights, etc., shall be repeated from sheet to sheet.

Holes for field connections must always be located independently, even if figured in connection with shop-rivets; they shall be repeated from sheet to sheet unless they are standard, in which case they shall be identified by a mark and the sheet given on which they are detailed.

A diagram in small scale, showing the relative position of the member in the structure, shall appear on every sheet. The member or members, which are detailed on the sheet, shall be shown in black, and the rest in red ink. Plain building work is exempt from this rule.

The quality of material, workmanship, size of rivets, etc., shall be specified on every sheet as far as it refers to the sheet itself. Standard workmanship, such as milling and tight fit of stiffeners, milling ends of columns, etc., etc., shall not be specified on drawings.

Each piece which is shipped separately shall have a shipping mark. These marks shall consist of capital letters and numerals, or numerals only; no small letters shall be used except when sub-marking becomes absolutely necessary. The letters R. and L. shall be used only to designate "right" and "left." Never use the work "marked" in abbreviated form in front of the letter, f. i., "3 Floorbeams, mk. G4;" say "3 Floorbeams G4."

Pieces which are shipped bolted on to a member shall, as a rule, also have a separate mark in order to identify them should they for some reason or another become detached from the main member. The drawing

Marking System. shall specify which pieces are to be bolted on for shipment, and the necessary bolts shall be billed.

A system of assembling marks shall be established for all small pieces in a structure which repeat themselves in great numbers. These marks shall consist of small letters and numerals or numerals only; no capital letters shall be used; avoid prime and sub-marks, such as m'a.

Lettering.

For all lettering use plain letters, see pages Nos. 49 to 57. For title, main dimensions and for all marks, particularly shipping marks, use heavy type. Red ink (Winsor & Newton's Carmine) shall be used for dimension, reference lines, etc.

Conventional Signs. Conventional signs for rivets are shown on page No. 18. Countersunk rivet heads project $\frac{1}{5}$; if less height of heads is required, drawings shall specify that they are to be chipped, or that they must not project more than $\frac{1}{16}$.

Flattened heads project from \$" to 7"; if less height of heads is required, they shall be countersunk.

Shop Bills.

Shop bills shall be written on special forms provided for the purpose. When the bills appear on the drawings as well, they shall either be placed close to the member to which they belong or on the right hand side of the sheet.

When the drawings do not contain any shop bills, these shall be so written that each sheet can have its bills attached to it, if desired; i. e., one page of shop bills shall not contain bills for two sheets of drawings.

In large structures, such as Elevated Railroads, Viaducts, etc., which always are subdivided into shipments of suitable size, both mill and shop bills must be written separately for each shipment.

In writing the shop bill, bear in mind that it shall serve as a guide for the laying out and assembling of the member, besides being a list of the material required. For this reason members which are radically different as to material shall not be bunched in the same shop bill, neither shall pieces which have different marks be bunched in the same item, even if the material is the same.

The main material in a member shall be billed first, followed by the smaller pieces. It is generally a good practice to begin at the left end of a girder, or at the bottom of a post or column. Do not bill first all the angles and then all the flats; when f. i. the end stiffeners in a girder are billed, the fillers belonging to them shall follow immediately after the angles, and so on. In a column each different bracket shall be billed complete by itself.

When machine-finished surfaces are required, the drawing and the shop bill shall specify the finished width and length of the piece, proper allowance for shearing and planing being made in mill bill. When the metal is to be planed as to thickness, the drawing and shop bill shall specify both the ordered and the finished thickness, f. i., one pl. 12" x 13" x 1' 6" planed to 3".

Flats and universal plates over 4" in width should be ordered in even inches; flats under 4" should be ordered by ½" variation in width. Flats ½" and under in thickness are very difficult to secure from the mills, and should be avoided if possible.

Sub-Divisions.

Every contract embracing different classes of work shall have a subdivision for each class. These subdivisions will be furnished by the Ch. Eng. of the district. Drawings, shop and shipping bills must be kept separate for each division.

Plate Girder Bridges.

General Rules.

As soon as a plate girder span is taken in hand, it shall be laid out in regard to location of web splices, stiffeners, coverplates, and in a through span, floor-beams and stringers, so that the material can be ordered at once if required.

Locate splices and stiffeners with a view of keeping the rivet-spacing as regular as possible; put small fractions at end of girder.

Stiffeners, to which cross-frames or floor-beams connect, must not be crimped, but shall always have fillers. The outstanding leg shall not be less than 4", gauged 2\frac{3}{4}"; this will enable cross-frames or floor-beams to be swung in place without spreading the girders.

The second pair of stiffeners at the end of girder over the bed-plate shall be placed so that the plate will project not less than 1" beyond the stiffeners.

Always endeavor to use as few sizes as possible for stiffeners, connection plates, etc., and avoid all unnecessary cutting of plates and angles. For this purpose locate end holes for laterals and diagonals so that they can be sheared in a single operation.

In spans on a grade, unless otherwise specified, put the necessary bevel in the bed-plate and not in the base-plate.

In short spans, say up to 50 feet, put slotted holes for anchor-bolts in both ends of girders.

In square spans show only one-half, but give all main dimensions for whole span.

In skew spans show whole span; when panels in one-half of span are same as in other half, give the lengths of these panels, but do not repeat rivet-spacing, except where it differs.

In the small scale diagram, which shall appear on every sheet, unless span is drawn in full, show the position of stiffeners, particularly those to which crossframes or floor-beams connect.

On top of sheet show top view of span, with cross- Deck Spans frames, laterals and their connections complete, the girders placed at right distance apart.

Below this view show the elevation of the far girder seen from inside, with all field holes in flanges and stiffeners indicated and blackened.

At one end of the elevation show in red the bridge-seat and back wall, give figures for distance from base of rail to top of masonry, notch of ties, depth of girder, thickness of base-plate and of bed-plate or shoe. When the other end of girder has a different height from base of rail to masonry, give both figures at the one end, and specify "for this end" and "for other end."

If span has bottom lateral bracing, a bottom view (horizontal section) shall be shown below the elevation.

When no bottom laterals are required, show only end or ends of lower flange of girder, giving detail of base-plate and its connection to the flange. Detail the bed-plate separately, never show it in connection with the base-plate.

Cross-frames shall, whenever possible, be detailed on the right hand of the sheet in line with the elevation. The frame shall be made of such depth as to permit it being swung into place without interfering with the heads of the flange-rivets in the girders. Always use a plate, not a washer with one rivet, at the intersection of diagonals.

In skew spans it is always preferable to have an even number of panels in the lateral system.

Through Spans.

Show on top of sheet an elevation of the far girder, seen from inside; below this view show a horizontal section of span, seen from above with lateral system detailed complete. It is generally best to show floor-beams and stringers in red in this view and detail them on a separate sheet.

The stiffeners in a through span should always be arranged so that the floor system can be put in place from the centre towards the ends.

What is said under "deck spans" about showing bridge-seat, back wall, detailing bed-plate separately, etc., applies to through spans as well.

Truss Bridges.

General Rules.

Before any details are started all c. to c. lengths of chords, posts, diagonals, etc., shall be determined, and sketches made of shoes, panel-points, splices, etc., so that the material can be ordered as soon as required.

If not otherwise specified, camber shall be provided in the top chord by increasing the length \(\frac{1}{8} \)" for every 10 feet. This increase in length shall not be considered in figuring the length of the diagonals, except in special cases, as directed by the engineer in charge. Half the increase in length shall be considered in figuring the length of top laterals.

Particular attention must be paid to what is said under "General Rules," on page No. 79, about showing part of adjoining member in red, and about small scale diagram on every sheet.

For every truss bridge an erection diagram shall be made on a separate sheet, giving the shipping marks of the different members and all main dimensions, such as c. to c. trusses, height of truss, number and length of panels, length of diagonals, distance from base of rail to masonry, from centre of bottom chord or pin to masonry, etc., etc. Give further size and number of bars in bottom chord and diagonals, size and grip of pins, and show in larger scale the packing at panel points. State also any special feature which the erector needs look out for, and give approximate weight of heavy and important pieces when their weight exceeds five tons. If in any place it is doubtful whether rivets can be driven in the field, the erection diagram and also the detail drawings shall state that "bolts may be used if rivets cannot be driven." A list giving number and contents of drawings belonging to the bridge shall also appear on the erection diagram sheet.

In square spans, not too large, show the left half of Lattice Bridges the far truss, seen from inside, and detail all members in their true position, making the skeleton one-half the scale of the details.

In skew spans, not symmetrical, show the whole of the far truss.

In large spans detail every member separately. When detailing web members bear in mind that the intersection point on the chord must not be used as a working point for a member which stops outside of the chord. A separate working point, preferably the end rivet, must be established on the member proper, and tied up with the intersection point on the chord.

The clearance between the chord and a web member entering same shall, whenever possible, not be less than $\frac{1}{8}$ " in heavy and $\frac{1}{16}$ " in light structures.

Members shall be marked with the panel points between which they go, f. i., End post LO-U1; 1st post L1-U1; top chord U1-U3, etc., etc. See diagram, page No. 50.

Pin-connected Bridges. In pin-connected bridges detail the left half of the far truss, every member by itself. It is generally best to commence with the end post, showing it lengthwise on the sheet with the lower end to the left, then the first section of the top chord, and so on.

The packing at panel points shall, whenever possible, be so arranged that, besides the customary allowance of $\frac{1}{16}$ for every bar, a clearance of not less than $\frac{3}{2}$ can be provided between the two sides of the chord. When more than two pin plates are used, $\frac{1}{32}$ should in addition be allowed for each plate.

Members shall be marked same as for lattice bridges, with the panel points between which they go, except the posts, which are best marked with letters and numerals. See diagram, page No. 50.

Office Buildings. Factories and Warehouses.

The different sheets shall be numbered consecu- Numbering of tively, whether large or small. No half numbers are permissible except in emergency cases. It is always well to arrange the numbers so that the sheets follow in the order in which the material is required at the building. The following is generally a good order:

Floor Plans for all floors, Column Schedule. Cast-iron Bases for Columns, Foundation Girders.

Beams.

First tier of Columns,

Riveted Girders, connecting to first tier of Columns, 66 66 Beams

Miscellaneous material for above,

Second tier of Columns, etc., etc.

Floor plans shall, as a rule, be made in scale \(\frac{1}{8}\)' to Floor Plans. the foot, see page No. 53. A separate plan shall be made for each floor, unless they are exactly alike.

Columns shall be marked consecutively with numerals, the word Col. always appearing in front of the numeral, f. i., "Col. No. 20." The architect or engineer has generally on his drawings adopted a system of marking for the columns, which should be adhered to, unless altogether too impracticable.

Riveted girders shall be indicated with two (2) fine lines when they have cover plates, and with four (4) fine lines when they have no cover plates. They shall be marked consecutively with numerals, using same marks for girders which are alike.

Beams and channels shall be indicated with one single heavy line. They shall be marked same as girders, with numerals, using same marks when alike.

Tie rods shall be indicated with one single fine line; they need not have any marks.

The marking system shall be as uniform as possible for the different floors, *i. e.*, a beam which goes between columns No. 2 and No. 3 shall be marked with the same numeral throughout all the floors.

All figures necessary for making the details shall, as a rule, appear on the floor plan, care being taken in writing same to leave room for the erection marks, which must be printed in heavy type above the line or lines representing a beam or girder.

Column Schedule For every large building a schedule of the columns shall be made before the details are started. See page No. 52.

Each column, even should several be alike, shall have a separate space, in which shall be given the material and finished length.

As soon as the detail drawings for one tier of columns are finished the sheet numbers shall be inserted as shown on sample schedule, making the schedule serve as an index for the column drawings.

Columns.

Columns shall, whenever possible, be drawn standing up on the sheet as they appear in the building. If it becomes necessary to draw them lengthwise on the sheet, the base shall be to the left.

Particular attention shall be paid to establishing a marking system for brackets, splice-plates, etc. A summary of all these standard pieces shall be made for each tier and sent to the shop as early as practicable, in order that they may be gotten out before the main material is

taken up. The material for the small pieces shall, as far as possible, be chosen from stock sizes.

Columns shall be marked with the number of the floor between which they go; f. i., Col. No. 4 (1-3). The lower tier is best marked "Basement Tier."

Girders shall be marked with the number of the floors, not with letters, unless specially requested; f. i., "2d Floor, No. 5."

Riveted Girders.

What is said under columns about marking system for standard pieces applies to girders as well.

When a girder is unsymmetrical about the centre line, and a question may arise how to erect it, one end of the same shall be marked with the number of the column to which it connects, or with North, South, East or West.

Girders must not be bunched together for the different floors more than to meet the requirements in the field; *i. e.*, they must correspond to the tiers of columns as they will be erected.

Reams

Beams shall be drawn on the standard forms provided for the purpose, see pages Nos. 54 to 57. They need not be drawn to scale; neat freehand sketches being allowed—in fact, desirable, where it will facilitate the rapid completion of the drawing without sacrificing clearness.

Beams shall be marked same as girders with the number of the floor; f. i., One 12" x 40 lbs. I x 19'- $3\frac{1}{2}$ ", Mark 2d Floor No. 35.

What is said under girders about marking one end, when not symmetrical around centre line, and about not bunching the different floors more than to meet the requirements in the field, applies to beams as well.

Whenever possible use standard framing angles.

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DEPARTMENT OF CIVIL ENGINEERING
PERKELEY, CALIFORNIA

If it is deemed necessary to use $6'' \times 6''$ angles, punch both legs same as 6'' leg of standard; in $3\frac{1}{2}'' \times 3\frac{1}{2}''$ or $4'' \times 3\frac{1}{2}''$ angles, punch both legs same as 4'' leg of standard. It is not absolutely imperative that the gauge of the framing angles shall be standard as long as the vertical distance between the holes and in the 6'' leg the horizontal distance $(2\frac{1}{2}'')$, is kept standard.

Holes for connections, tie-rods, etc., shall be located from one end of the beam, preferably the left. If one end rests on the wall and the other end is framed, then figure from the latter end, be it right or left. This rule may be dispensed with in case of numerous holes regularly spaced in web or flange for connection of shelf-angles, buckle-plates, etc.

The allowed overrun at ends of beams must always be indicated, either by giving figures or by showing wall bearing.

Holes at end of beam for anchors are best figured from wall end, not connecting them with other figures.

The distance between end holes in beams which connect through web or flange to columns, girders, etc., shall always be given.

When framing angles are standard, do not give any figures for either shop or field rivets, except the distance from bottom of beam to centre of connection or to first hole in framing angle, and the horizontal distance between field holes.

When special framing angles are used, the fact must be noted and figures given for gauges, etc.

For standard connection holes in web of beam all figures required are the distance from bottom of beam to centre of connection or to first hole and the horizontal distance between holes. Whenever possible use standard punching as given on pages Nos. 9 and 10.

APPENDIX

TO

RULES FOR MAKING SHOP-DRAWINGS.

Two methods may be employed in making shop-drawings for trusses in Mill Buildings or other structures, and for lattice girders in bridges.

The First Method is to make the drawings so complete (see previous rules) that the templets can be made for each individual piece separately on the bench.

The Second Method is to give on the drawings only sufficient dimensions to locate the interior of the member and the position of all pieces, leaving the details to be worked out by the templet-maker on the laying-out floor.

Sufficient figures should be given to definitely establish the main laying-out points; generally these figures should be those locating the outside dimensions of the chord of a truss, the end depths or such heights as may be necessary to establish the general outside lines of the complete member.

The interior pieces should be located by centre-lines corresponding to the gage lines of angles, or the centre of gravity lines of the pieces, as the case may be.

The rivet-spacing should be given complete for all connections to members not shown on the same sheet, in places where it becomes necessary to indicate clearance on opposite flanges, and for any connections which may be readily located from fixed points without employing any computation.

All other rivet-spacing, such as the connections of web pieces

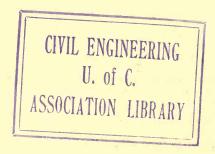
to gussets, and the lengths of interior pieces, may be indicated by scale, leaving the actual location of the rivets to be determined by the templet-maker. The drawing should, however, indicate the number of rivets to be used in each individual connection, and should also state the usual rivet pitch to be employed for the work shown on the sheet, as well as the minimum rivet-spacing allowed.

No definite rule can be laid down as to which method should be employed; but in general straight work, such as columns, plate girders, heavy lattice girders in buildings and chords, floor beams, and stringers in highway bridges, should be laid out by the first method.

All roof trusses, light lattice girders and complicated work, such as towers, domes, hips and light lattice struts, should be laid out by the second method.

Before deciding which of the two methods should be employed in any individual case, the templet-maker should be consulted as to the facilities for undertaking the work, as lack of floor space in his shop may prevent the use of the second method.





Points to be Observed in Order to Facilitate Erection.

The first consideration for ease and safety in erection should be to so arrange all details, joints and connections that a structure may be connected, made self-sustaining and safe in the shortest time possible.

Entering connections of any character should be avoided when possible, notably on top chords, floor beam, and stringer connections, splices in girders, etc., etc.

When practicable, joints should be so arranged as to avoid having to put members together by entering them on end, as it is often impossible to get the necessary clearance in which to do this.

In all through spans floor connections should be so arranged that the floor system can be put in place after the trusses or girders have been erected in their final position, and *vice versa*, so that the trusses or girders can be erected after the floor system has been set in place.

All lateral bracing, hitch-plates, rivets in laterals, etc., should, as far as possible, be kept clear of the bottom of the ties, it being very expensive to cut out ties to clear such obstructions.

Lateral plates should be shipped loose, or bolted on so that they do not project outside of the member, whenever there is danger of them being broken off in unloading and handling.

Loose fillers should be avoided. They should be tacked on with rivets, countersunk where necessary.

In elevated railroad work, viaducts and similar structures, where longitudinal girders frame into cross girders, shelf angles should be provided on the latter. In these structures the expansion joints should be so arranged that the rivets connecting the fixed span to the cross girder can be driven after the expansion span is in place.

In viaducts, etc., two spans, abutting on a bent, should be so arranged that either span can be set in place entirely independent of the other. The same thing applies to girder spans of different depth resting on the same bent.

Holes for anchor-bolts should be so arranged that the holes in the masonry can be drilled and the bolts put in place after the structure has been erected complete.

In structures consisting of more than one span a separate bed-plate should be provided for each shoe. This is particularly important where an old structure is to be replaced; if two shoes were put on one bed-plate or two spans connected on the same pin, it would necessitate removing two old spans in order to erect one new one.

In pin-connected spans the sections of top chords nearest the centre should be made with at least two pin-holes. In skew spans the chord splices should be so located that two opposite panels can be erected without moving the traveler.

Tie plates should be kept far enough away from the joints, and enough rivets should be countersunk inside the chord to allow eyebars and other members being easily set in place.

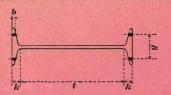
Posts with channels or angles turned out and notched at the ends should, whenever possible, be avoided.

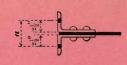
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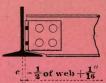
APPENDIX

Shapes rolled by Carnegie Steel Company

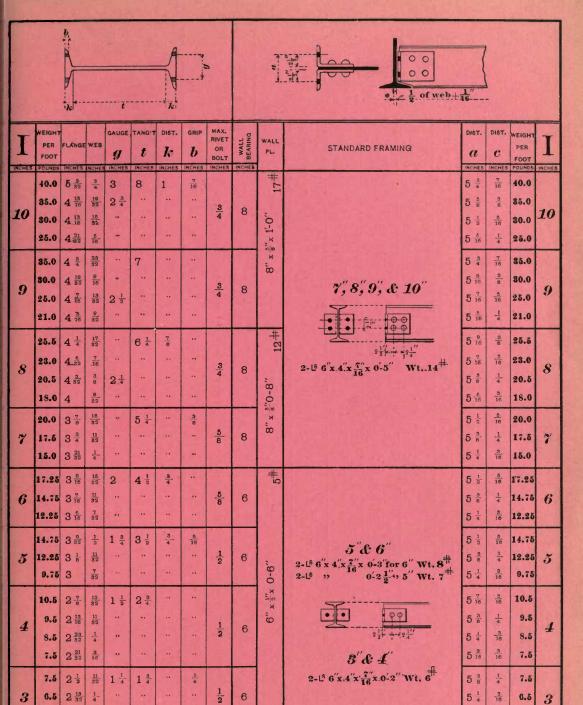
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All rivets in standard framing angles are $\frac{3}{4}$ diam.

Weights of " " Include weight of shop rivets only.

When beams frame opposite each other into another beam with web thickness less than $\frac{9''}{16}$ or where beams of short span lengths are loaded to their full capacity, is may be necessary to use framing angles of greater strength than the standards.

See table below for minimum span lengths.

5.5 2 5 33

I	WEIGHT	SPAN IN	I	WEIGHT	SPAN IN	Ι	WEIGHT	SPAN IN	I	WEIGHT	SPAN IN	I	WEIGHT	SPAN IN	I	WEIGHT	SPAN IN	I	WEIGHT	SPAN IN
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20	80.0	22.0	18	55.0	14.0		60.0	15.5	12	40.0	11.5	10	25.0	9.0	7	15.0	4.0	4	7.5	3.0
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10	35.00 30.00 25.00 20.00 15.00	$ \begin{array}{c} 3\frac{5}{32} \\ 3\frac{1}{32} \\ 2\frac{7}{8} \\ 2\frac{23}{32} \\ 2\frac{19}{32} \end{array} $	13 18 11 18 17 32 3 8	11/2	8½	7 8 6.6 6.6 6.6 6.6	66 66 7 18 66	3 4	11/4	$ \begin{array}{c c} 2\frac{1}{16} \\ 1\frac{15}{16} \\ 1\frac{25}{32} \\ 1\frac{5}{8} \\ 1\frac{1}{2} \end{array} $	$3\frac{5}{16}$ $3\frac{8}{16}$ $3\frac{1}{16}$ $2\frac{7}{8}$ $2\frac{3}{4}$	7 8 3 4 5 8 7 110 5 110	35.00 30.00 25.00 20.00 15.00	10
9	25.00 20.00 15.00 13.25	$ \begin{array}{c} 2\frac{13}{16} \\ 2\frac{21}{32} \\ 2\frac{1}{2} \\ 2\frac{7}{16} \\ 2\frac{5}{8} \end{array} $	5 8 7 16 9 32 7 32 7 32	13 18	7 ¹ / ₄	7. B 4.4 4.4	€. €. €.	3/4	1 1/8 · · · · · · · · · · · · · · · · · · ·	$ \begin{array}{c} 1\frac{3}{4'} \\ 1\frac{6}{16} \\ 1\frac{13}{32} \\ 1\frac{11}{32} \\ 1\frac{21}{32} \end{array} $	$3\frac{1}{6}$ $2\frac{15}{16}$ $2\frac{13}{18}$ $2\frac{3}{4}$ $3\frac{1}{16}$	11/16 1/2 3/8 5/16	25.00 20.00 15.00 13.25 21.25	9
8	21,25 18.75 16.25 13.75 11.25	$2\frac{17}{32}$ $2\frac{17}{32}$ $2\frac{7}{16}$ $2\frac{11}{32}$ $2\frac{1}{4}$	18 13 32 5 18 7	1 ₃ 1 ₈	04 66 66	6.6 6.6 6.6	6.6 6.6 6.6 6.6	3)4	## ## ### ### ### ### ### ### ### #### ####	1 $\frac{9}{16}$ 1 $\frac{15}{32}$ 1 $\frac{3}{8}$ 1 $\frac{9}{32}$	$egin{array}{c} 3 & & & & & & \\ 3 & & & & & & \\ 2^{7/8}/8 & & & & & \\ 2^{13/16}/6 & & & & & \\ 2^{3/4}/6 & & & & & \\ & & & & & \\ & & & & & \\ & & & & & \\ & & & & & \\ & & & & & \\ & & & & & \\ & & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & & \\ & & \\ & & & \\ $	9 18 7 16 3 8 5	18.75 16.25 13.75 11.25	8
7	19.75 17.25 14.75 12.25 9.75	$ \begin{array}{c} 2\frac{1}{2} \\ 2\frac{13}{32} \\ 2\frac{5}{18} \\ 2\frac{3}{.18} \\ 2\frac{3}{32} \end{array} $	5 8 17 32 7 16 5 18 7 32	1 1 2	5 ⁴ (2)	3 4 6,6 6,6	66 66 66	<u>5</u> 8	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	$ \begin{array}{c} 1\frac{11}{16} \\ 1\frac{19}{32} \\ 1\frac{1}{2} \\ 1\frac{3}{6} \\ 1\frac{9}{32} \end{array} $	$3\frac{1}{8}$ $3\frac{1}{10}$ $2\frac{15}{10}$ $2\frac{13}{10}$ $2\frac{3}{4}$	11 18 0 18 1 1 2 8 8	19.75 17.25 14.75 12.25 9.75	7
6	15.50 13.00 10.50 8.00	$ \begin{array}{c} 2\frac{9}{32} \\ 2\frac{5}{32} \\ 2\frac{1}{32} \\ 1\frac{29}{32} \end{array} $	9 16 7 16 5 16 3 15	" " 11	41/2	6.6 6.6 6.6	6.6 6.6 6.6	<u>5</u>	15 16 6,6 6.6	1½ 138 1¼ 1¼ 1½	$3\frac{1}{16}$ $2\frac{15}{16}$ $2\frac{13}{18}$ $2\frac{11}{16}$	5 8 1 2 3 8	15.50 13.00 10.50 8.00	6
5	9.00 6.50	$ \begin{array}{c c} 2\frac{1}{32} \\ 1\frac{29}{32} \\ 1\frac{3}{4} \end{array} $	15 32 11 32 3 18	e e e e	3 4	5 8	16 11	1/2	7 8 4 f	$1\frac{1}{32}$ $1\frac{7}{32}$ $1\frac{1}{16}$	$3 \\ 2^{\frac{13}{16}} \\ 2^{\frac{11}{16}}$	9 16 3 8	9.00 6.50	5
4	7.25 6.25 5.25	$1\frac{23}{32}$ $1\frac{31}{32}$ $1\frac{19}{32}$	5 16	1	234	.5 8 11	66	1 2	3 4 **	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	$2^{\frac{13}{16}}$ $2^{\frac{3}{4}}$ $2^{\frac{11}{16}}$	3 8 5 18 14	7,25 6.25 5.25	4
3	6.00 5.00 4.00	1\frac{5}{8} 1\frac{1}{2} 1\frac{13}{32}	3 8 14 5 32	7 8 6	134	.5 8 4.6	1 4 66	1 2	5-18 11	1 -7 8 23 33	$2\frac{7}{8}$ $2\frac{3}{4}$ $2\frac{11}{16}$	7 18 5 16 1 4	6.00 5.00 4.00	3



All dimensions in inches WEIGHT AREA ACTUAL SIZE GAUGE MAX. RIVETS NOMINAL SIZE GAUGE NOMINAL THICKNESS THICKNESS PER SIZE G1 G^1 FLANGES & WEB G \boldsymbol{G} FOOT INCHES 21 x 3 x 216 6.7 1.97 15 1 2 14 18 5 16 $2\frac{3}{4} \times 3\frac{1}{10} \times 2\frac{3}{4}$ 8.4 2.48 38 38 21 × 3 × 21 9.7 2.86 $\frac{7}{16}$ 7 23 x 31 x 23 3.36 11.4 3 3 12 21 × 3 × 21 × 12.5 3.69 12 18 9 16 $2\frac{3}{4} \times 3\frac{1}{16} \times 2\frac{3}{4}$ 14.2 4.18 1 34 78 316 x 4 x 318 4 8.2 2.41 2 2 5 16 5 16 $3\frac{1}{8} \times 4\frac{1}{16} \times 3\frac{1}{8}$ 10.3 3.03 .. 38 38 3 x 4 x 3 x 3 x 12.4 3.66 7 7 16 3 18 × 4 × 3 18 13.8 4.05 12 1 2 3 x 4 18 x 3 8 15.8 4.66 9 9 18 $3\frac{3}{16} \times 4\frac{1}{3} \times 3\frac{3}{16}$ 17.9 5.27 4 4 58 5 31 × 4 × 31 8 5.55 18.9 11 11 $3\frac{1}{8} \times 4\frac{1}{10} \times 3\frac{1}{8}$ 20.9 6.14 3 34 3 x 4 x 3 x 3 x 22.9 6.75 5 16 78 5 16 78 $3\frac{1}{4} \times 5 \times 3\frac{1}{4}$ 11.6 3.40 21 21 38 38 $3\frac{5}{16} \times 5\frac{1}{16} \times 3\frac{5}{16}$ 13.9 4.10 " . . 716 7 16 33 x 51 x 33 1.6.4 4.81 12 12 $3\frac{1}{4} \times 5 \times 3\frac{1}{4}$ 17.8 5.25 16 9 16 $3\frac{5}{10} \times 5\frac{1}{10} \times 3\frac{5}{10}$ 20.2 5.94 58 5 $3\frac{3}{8} \times 5\frac{1}{8} \times 3\frac{3}{8}$ 22.6 6.64 5 6.6 11 11 16 $3\frac{1}{4} \times 5 \times 3\frac{1}{4}$ 23.7 6.96 3 34 35 x 51 x 35 26.0 7.64 13 13 $3\frac{3}{8} \times 5\frac{1}{6} \times 3\frac{3}{8}$ 28.3 8.33 66 38 78 38 $3\frac{1}{2} \times 6 \times 3\frac{1}{2}$ 78 15.6 4.59 24 3 7 7 3 % × 6 1 × 3 18 5.39 18.3 12 12 35 x 65 x 35 21.0 6.19 9 18 9 $3\frac{1}{2} \times 6 \times 3\frac{1}{2}$ 22.7 6.68 5 8 58 3 x 6 1 x 3 18 25.4 7.46 분 6 35 x 61 x 35 28.0 8.25 11 16 6 3 34 $3\frac{1}{3} \times 6 \times 3\frac{1}{2}$ 29.3 8.63 13 3 x 6 x 3 x 13 32.0 9.40 7 8 78 $3\frac{5}{3} \times 6\frac{1}{8} \times 3\frac{5}{8}$ 34.6 10.17

WEIGHTS OF ANGLES

All dimensions in Inches

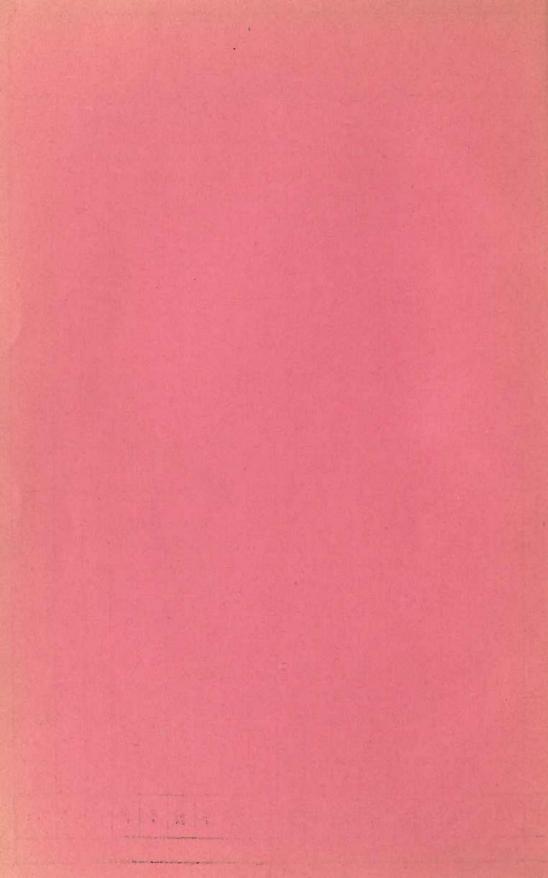
							A	.ll dime	nslons	in Inch	es	100	15.79	Holi	The i		100	
SIZE	1 8	$\frac{3}{16}$	1/4	$\frac{5}{16}$	3/8	7 16	1/2	9 76	5/8	11 16	3 4	13 16	7 8	15 16	1	11/16	11/8	SIZE
8 x 8					N.		26.4	29.5	32.7	35.8	38.9	42.0	45.0	48.0	51.0	54.0	56.9	8 x 8
6 × 6					14.8	17.2	19.6	21.9	24.2	26.5	28.7	30.9	33.1	35.3	37.4			6 * 6
* 5 × 5					12.3	14.3	16.2	18.1	20.0	21.8	23.6	25.4	27.2	28.9	30.6			5 * 5
4 × 4				8.2	9.8	11.3	12.8	14.3	15.7	17.1	18.5	19.9						4 × 4
$3\frac{1}{2} \times 3\frac{1}{2}$		-33		7.1	8.5	9.8	11.1	12.3	13.6	14.8	16.0	17.1						$3\frac{1}{3} \times 3\frac{1}{2}$
3 × 3			4.9	6.1	7.2	8.3	9.4	10.4	11.4					378				3 × 3
$2\frac{3}{4} \times 2\frac{3}{4}$	198		4.5	5.5	6.6	7.6	8.5											$\overset{*}{2}$ × $2\frac{3}{4}$
$\begin{array}{ccc} 2\frac{1}{2} \times 2\frac{1}{2} \\ \times \end{array}$		3.1	4.0	5.0	5.9	6.8	7.7											2½ × 2½
$2\frac{1}{4} \times 2\frac{1}{4}$		2.8	3.7	4.5	5.3	6.1	6.8								E-13			$\frac{\pi}{24} \times 2\frac{1}{4}$
2 × 2		2.5	3.2	4.0	4.7	5.3											44	2 × 2
$1\frac{3}{4} \times 1\frac{3}{4}$		2.1	2.8	3.4	4.0	4.6											ğ	$1\frac{3}{4} \times 1\frac{3}{4}$
$1\frac{1}{2} \times 1\frac{1}{2}$		1.8	2.4	2.9	3.4													$1\frac{1}{2} \times 1\frac{1}{2}$
$1\frac{1}{4} \times 1\frac{1}{4}$	1.0	1.5	1.9	2.4														1 × 1 × 1 ×
1 x 1	0.8	1.2	1.5															1 × 1
SIZE	1/8	3 16	1/4	<u>5</u>	3/8	7 16	1/2	9 16	5/8	<u>11</u> 16	3 4	13 16	7/8	15 16	1	11/16	13	SIZE
$\overset{*}{7}$ × $3\frac{1}{2}$						15.0	17.0	19.0	21.0	23.0	24.9	26.8	28.7	30.5	32.3	17/		* × 3½
6 × 4			Str		12.3	14.3	16.2	18.1	20.0	21.8	23.6	25.4	27.2	28.9	30.6			6 × 4
6 x 3½					11.7	13.5	15.3	17.1	18.9	20.6	22.3	24.0	25.7	27.3	28.9			$6 \times 3^{\frac{1}{2}}$
5 × 4					11.0	12.8	145	16.2	17.8	19.5	21.1	22.6	24.2					* × 4
$5 \times 3^{\frac{1}{2}}$				8.7	10.4	12.0	13.6	15.2	16.8	18.3	19.8	21.3	22.7					$5 \times 3^{\frac{1}{2}}$
5 × 3				8.2	9.8	11.3	12.8	14.2	15.7	17.1	18.5	19.9						5 × 3
$\overset{*}{4} \times 3^{\frac{1}{2}}$				7.7	9.1	10.5	11.9	13.3	14.6	15.9	17.2	18.5						$\overset{*}{4} \times 3^{\frac{1}{2}}$
4 × 3				7.1	8.5	9.8	11.1	12.3	13.6	14.8	16.0	17.1						4 × 3
3½ x 3			68	6.6	7.8	9.1		11.4				15.7					PEN.	$3\frac{1}{2} \times 3$
$3\frac{1}{2} \times 2\frac{1}{2}$	Jac.		4.9	6.1	7.2	8.3		10.4	11.4	12.4								$3\frac{1}{2} \times 2\frac{1}{2}$
$3 \times 2\frac{1}{2}$			4.5	5.5	6.6	7.6	8.5	9.5										$3 \times 2\frac{1}{2}$
3 × 2	10		4.0	5.0	5.9	6.8	7.7								Mr.		17 12	3 × 2
2½ x 2		2.8	3.7	4.5	5.3	6.1	6.8											$2\frac{1}{2} \times 2$
SIZE	1/8	3 16	1/4	<u>5</u>	3/8	7 16	1 2	9 16	5 8	11 16	3 4	<u>13</u> 16	7/8	15 16	1	116	18	SIZE
EVS AV	Angl	es m	arke	*	are s	necia	1			9718		377				188	ARE	

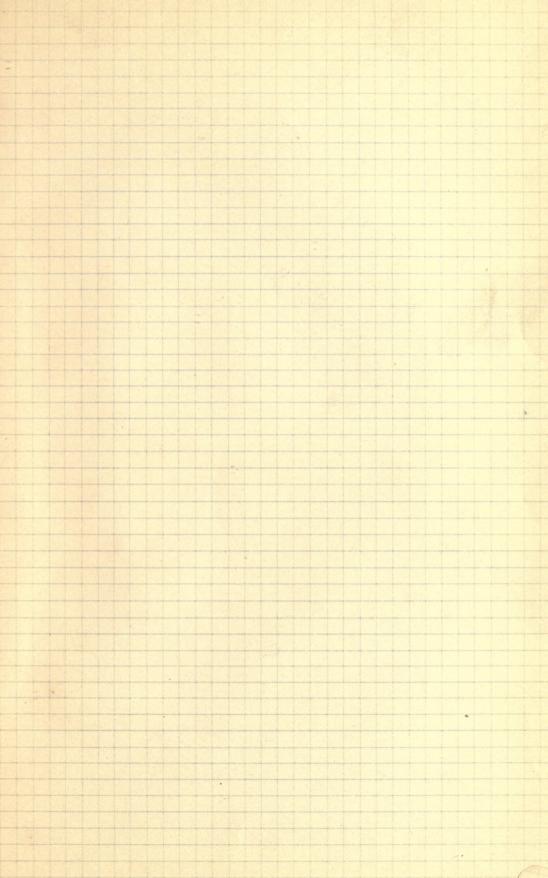
Angles marked * are special

ANGLES

Area in square inches,

SIZE	1/8	3 16	1/4	<u>5</u>	3/8	7 16	1/2	9 16	<u>5</u>	<u>11</u> 16	34	13 16	3	15 16	1	116	13	SIZE
8 x 8							7.75	8,68	9.61	10.53	11.44	12.34	13.23	14.12	15.00	15.87	16.73	8 x 8
6 × 6					4.36	5.06	5.75	6.43	7.11	7.78	8.44	9.09	9.74	10.37	11.00			6 × 6
5 × 5					3.61	4.18	4.75	5.31	5.86	6.42	6.94	7.46	7.99	8.50	9.00			* 5 × 5
4 × 4				2.40	2.86	3.31	3.75	4.18	4.61	5.03	5.44	5.84			M.E.			4 × 4
$3^{1}_{2} \times 3^{1}_{2}$				2.09	2.48	2.87	3.25	3.62	3.98	4.34	4.69	5.03				a X i		$3\frac{1}{2} \times 3\frac{1}{2}$
3 × 3			1.44	1.78	2.11	2.43	2.75	3.06	3.36									3 × 3
$2^{3}_{4} \times 2^{3}_{4}$			1.31	1.62	1.92	2.22	2.50											*23 x 23
$2\frac{1}{2} \times 2\frac{1}{2}$		0.90	1.19	1.47	1.73	2.00	2.25											$2\frac{1}{2} \times 2\frac{1}{2}$
$\overset{*}{2}_{\frac{1}{4}} \times 2_{\frac{1}{4}}$		0.81	1.06	1.31	1.55	1.78	2.00											$2\frac{1}{4} \times 2\frac{1}{4}$
2 × 2		0.72	0.94	1.15	1.36	1.56												2 × 2
$1\frac{3}{4} \times 1\frac{3}{4}$		0.62	0.81	1.00	1.17	1.30						186						$1\frac{3}{4} \times 1\frac{3}{4}$
$1\frac{1}{2} \times 1\frac{1}{2}$	0.36	0.53	0.69	0.84	0.99							5-5-						$1\frac{1}{2} \times 1\frac{1}{2}$
$1\frac{1}{4} \times 1\frac{1}{4}$	0.30	0.43	0.56	0.69								100						$1\frac{1}{4} \times 1\frac{1}{4}$
1 × 1	0.24	0.34	0.44				SV.					1 (4)						1 × 1
SIZE	1/8	3 16	1/4	<u>5</u>	38	7 16	1/2	9 16	<u>5</u>	11 16	3/4	13 16	78	1 <u>5</u>	1	116	13	SIZE
*7 × 3½						4.40	5.00	5.59	6.17	6.75	7.31	7.87	8.42	8.97	9.50			* 7 x 3½
6 × 4					3.61	4.18	4.75	5.31	5.86	6.41	6.94	7.47	7.99	8.50	9.00			6 × 4
6 x 3½					3.42	3.97	4.50	5.03	5.55	6.06	6.56	7.06	7.55	8.03	8.50			6 x 3½
* 5 × 4					3.23	3.75	4.25	4.75	5.23	5.72	6.19	6.65	7.11					* × 4
$5 \times 3\frac{1}{2}$				2.56	3.05	3.53	4.00	4.47	4.92	5.37	5.81	6.25	6.67					5 × 3½
5 x 3				2.40	2.86	3.31	3.75	4.18	4.61	5.03	5.44	5.84						5 × 3
* 4 x 3½				2.25	2.67	3.09	3.50	3.90	4.30	4.68	5.06	5.43						* 4 × 3½
4 × 3				2.09	2.48	2.87	3.25	3.62	3.98	4.34	4.69	5.03						4 × 3
3½ x 3		150		1.93	2.30	2.65	3.00	3.34	3.67	4.00	4.31	4.62						3½ x 3
$3\frac{1}{2} \times 2\frac{1}{2}$			1.44	1.78	2.11	2.43	2.75	3.06	3.36	3.65				61-18				$3\frac{1}{2} \times 2\frac{1}{2}$
3 x 2½		==19	1.31	1.62	1.92	2.22	2.50	2.78				5 V		7				3 x 2½
3 × 2			1.19	1.47	1.73	2.00	2.25	15,70	1	11/2								3 × 2
2½ x 2		0.81	1.08	1.31	1.55	1.78	2.00											2½ × 2
SIZE	18	3 16	1/4	<u>5</u>	38	7 16	1/2	9 16	<u>5</u>	11 16	34	13 16	7/8	15 16	1	116	11/8	SIZE
	Angl	les m	arke	d * a	re sp	ecial			13%			116		1				





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